

# <u>City of Bay Minette</u>

# **Site Plan Review Application**

Office Use Only Case No.:<u>5P.24002</u> Fee: \$500.00 Paid: □ Cash □ Check ■ Credit Card

301 D'Olive Street · Bay Minette, Alabama 36507 Phone (251) 580-1650 · COBM\_ Planning@cityofbayminetteal.gov

Are you the property owner? 
☐ Yes 
B No

(If you are not the property owner, you must submit an Agent Authorization Form signed by the property owner)

Applicant Name: Plasmine Technology (Bryan	Wiggins)	Date:_1/24/24
Mailing Address: 251 Newport Parkway		
City: Bay Minette	State: AL	Zip Code: <u>36507</u>
Phone Number: 251-937-2771	Email: _br	yanwiggins@plasmine.com
	Site Information	
Property Address: 251 Newport Pkwy		
or Property Location:		
*Parcel No.: 23-02-10-3-000-029.000 & 23-02-10	-3-000-082.000	*PPIN No.: _35534 & 62861
*Parcel or PPIN information must be complete	ed	
Request: Any necessary permitting for addition of t	anks and parking area.	
I, the undersigned, do hereby request the for the location to determine if it meets I understand and authorize City Staff to	ne City of Bay Minette Planni the regulations of the <i>Zoning</i> conduct site visits, as needed	ing Commission to grant a Site Plan Review g <i>Ordinance</i> for the reason(s) stated above. d in relation to this request.
Bryan Wiggins	Digitally signed by Bryan Wiggins Date: 2024.01.25 14:44:37 -06'00'	1/25/24
Signature of Applicant (Owner of Prop	erty of Authorized Agent)	Date
Submittal Requirements         Application         Fee paid in full         Agent Authorization Form (if app         Complete Legal Description of Pr	licant is not the owner) operty	x 2
Plot Plan or Survey – indicating a	ny existing structures, propose	d structures, and setbacks from property lines

\_\_\_\_\_ Submittal Requirements listed in 8.9 Site Plan, as applicable





8005 Morris Hill Road Semmes, AL 36575 Phone (251) 649-4011 Fax (251) 645-0971

#### Plasmine Technology, Inc. Trailer Storage & Off-Street Parking Drainage Narrative

#### **Existing Site Conditions:**

Plasmine Technology, Inc. is located at 251 Newport Pkwy, Bay minette, AL. The proposed trailer storage area will be located in the southeast corner of the property along the north right of way of Newport Pkwy. Currently there is a 0.35-acre asphalt parking lot located in the southeast corner of the property. The total pre-development drainage area is approximately 5.18 acres. Roughly 2.46 acres are currently wooded, and 1.2 acres are covered in heavy grass. The area currently drains south to Newport Pkwy right of way. See Drainage Area maps in Appendix E.

#### **Proposed Site Development:**

The site is intended to be used as a tanker trailer storage and off-street parking area. All construction will be at grade level with no vertical construction. A 1.6-acre concrete surface will be provided for the tanker trailer storage area. A 0.4-acre asphalt parking lot will be provided for office parking. All concrete parking surface will be contained within the interconnected north and south ponds and discharge will be controlled by slide gate valves to decrease the risk of contamination in case of tanker trailer spillage.

Primary site access during construction and operation will be from Newport pkwy, a paved, public right of way serving several industrial facilities, near and adjacent to this project.

The developed areas will be fenced and gated to limit access onto the site.

Two storm water retention ponds, North Pond and South Pond, will be constructed. To meet SPCC recommendations these should be sized to contain the 25-YR, 24 HR storm runoff from the area they will receive storm water from. However, these ponds have been designed to contain the calculated 100-Year, 24-Hour storm runoff. The ponds will be interconnected via a 24" diameter reinforced concrete pipe with a slide gate valve in the north pond outfall structure. The south pond outfall structure will include an additional slide gate valve to prevent discharge from both the north and south ponds during rain events. These valves will only be opened after impounded storm water has been tested and is found to be free of potential chemical leakage. The outfalls of both ponds will discharge water to a riprap pad adjacent to the existing Newport Pkwy right of way once the valves are opened.

A smaller storm water detention pond, Office Parking lot pond will be constructed to meet the stormwater detention requirements for the office parking lot. The pond outfall will also drain to the riprap pad adjacent to the existing Newport Pkwy right of way separate from the north and south pond outfall.

See Drainage Area maps in Appendix E.

#### **Methodology:**

Methodology used to assess the hydrology and hydraulics of the existing and proposed conditions are considered regionally acceptable by multiple governing jurisdictions within the state of Alabama. They are as follows:

• Modified Rational Method for converting rainfall to runoff (Appendix D: Driven Engineering Inc. Calculation Spreadsheets).

• TR55 for calculating retention pond volume. (See appendix C for calculations)



#### **Results:**

The results of the analysis are provided as appendices of this narrative; however, we have summarized them below: The drainage area map was used to determine a drainage area based on contours using topographic survey data.

#### Pre- and Post- Development Runoff:

The Pre-Development drainage area to the subject outfall location including portions of the site to not be developed is 5.18-acres. The calculated pre-development runoff for a 10-yr storm is 13.32 cfs, and a 100-yr return period storm is 16.75 cfs. (see page 6 for more details) The outfall pipe size for the containment/detainment ponds was chosen to be a 15" to throttle the flow once the valves are opened. The parking lot pond flow would be finished by the time the valves are opened. During a storm event up to a 100-yr storm, the calculated post-development discharge will be no more than 4.24 cfs from the parking lot pond only (see page 8). After the 100-yr storm event is over, the off street parking pond outflow should be finished, and after testing the water the site's gate valves may be opened from the larger ponds. At that point no more than 4.96 cfs will be discharging, which is manning's full flow from a 15" pipe @ 0.5%, still less than calculated pre-development peak discharge. All of this is designed to enter a riprap stilling basin before exiting the site to the current discharge area in the existing ROW. At no time will calculated post-development peak flows exceed 40% of pre-development peak flows. Appendix D, Page 7 calculates a likely never to be needed emergency overflow for the two large ponds that assumes if all ponds have filled up due to a 100-year rain event and another 100-year event occurs, the overflow weir will need to be roughly 16 ft wide at the available depth, to prevent a blow-out of the pond berms.

#### North and South Pond:

Drainage area 1 is 2.57 +/- AC for the north & south detention ponds. The current use of the land is wooded and grassy. After construction, 1.45-acres will be paved parking and trailer storage and 0.9-acres used for detention, with the remaining area (0.22 acres) being grassed. The pond volume was checked using a depth of 10 inches of rain per the SCS rainfall intensity map for Bay Minette, AL for a 25-year, 24- hour storm (Appendix A, Fig B-6.) The volume was also checked for the 12-inch rainfall depth acquired from the 100-year, 24-hour storm (Appendix B, Fig B-8.) Regardless of the finished surfaces within this area we have assumed no infiltration and no evaporation for a very conservative estimate of needed containment volume. See Appendix C, page 5 for volume calculations.

#### **Office Parking Pond:**

Drainage area 2 is 0.90 +/- AC for the office parking lot pond. The current use of the land is mostly grass, with a 0.35-acre asphalt parking lot. After construction, 0.38-acres will be asphalted for office parking with the rest remaining grassy area. The pond volume was calculated for a 50-year storm being detained and limiting the outflow to a 10-yr storm release rate as required by the City of Bay Minette. (See Appendix D: Driven Engineering Inc. Modified Rational Method Drainage Calculation Spreadsheet, on page 8).

The point of discharge will be armored with riprap for energy dissipation and erosion control.

Prepared by Nathan Williams, EI

Approved by Avalisha Fisher, P.E.



#### Appendix A: 25-YR, 24-HR Rainfall Chart





Figure B-6: 25-Year, 24-Hour Rainfall



#### Appendix B: 100-YR, 24-HR Rainfall Chart





Figure B-8: 100-YR, 24 -HR Rainfall



#### Appendix C: TR-55 Pond Volume Calculations

Assumptions: No Infiltration and No evaporation, every drop of rain will be stored in the detention ponds:

Drainage Area 1 = 2.57 AC

25-YR, 24-HR Rainfall from Figure B-6 Appendix A for Bay Minette, AL = 10" 100-YR, 24-HR Rainfall from Figure B-8 Appendix B for Bay Minette, AL = 12"

> Pond Volume Required for 25 YR Storm = (2.57 AC) x (43,560 SF) x (0.83 FT) = 92,900 CF

Pond Volume Required for 100 YR Storm = (2.57 AC) x (43,560 SF) x (1 FT) = 112,000 CF

At Emergency Overflow EL: 262.25 North Pond Volume = 54,851 CF South Pond Volume = 61,114 CF Total Pond Volume = 115,965 CF > 100 YR STORM EVENT RUNOFF



## Appendix D: Driven Engineering Inc. Modified Rational

## **Method Drainage Calculation Spreadsheets**

Driven Engineering, Inc.							ngineering.cor	<u>n</u>
	DDING VAL	UE, BUILDING	RELATIONS	riir3				
8005 Morris Hill	Road							
Semmes, AL 3	6575			Project Number			23105	
(251)649-4011								
				Date:			2/29/2024	
PROJECT NAM	E							
Plasmine Prede	velopment C	Outfall Location	Runoff					
Runoff Coeffici	ent Determ	ination						
	Pre-Develop	oment						
Area	Acres	Coefficient	C*A					
Exist. imPerviou	0.35	0.9	0.32					
pervious	4.83	0.25	1.21					
semi	0.00	0.00	0.00					
Total Area	5.18							
Weighted Avera	ge=	0.29	1.52					
Pre and Post Flow Data For 10, 50 and 100 Year Storms In CFS:								
(Note: Post runoff values represent flow to detention, not flow from the site.)								
Q 10 Pre=	13.32	Q 10 Post=	0.00		110	8.75	in/hr	
Q 25 Pre=	13.85	Q 25 Post=	0.00		125	9.10	in/hr	
Q 50 Pre=	15.38	Q 50 Post=	0.00		150	10.10	in/hr	
Q 100Pre=	16.75	Q 100 Post=	0.00		1100	11.00	in/hr	



	Driven	Fnainee	rina. li	nc		www.drivene	ngineering.co	<u>m</u>	
	2111011	Linginioe	,	10.					
	ADDING VAL	UE, BUILDING	RELATIONS	HIPS					
8005 Morris Hill	Road								
Semmes, AL 3	6575			Project N	lumber		23105		
(251)649-4011									
				Date:			3/5/2024		
PROJECT NAM	IE								
EMERGENCY	OVERFLOW	/ FOR ENTIRE	AREA assu	ımes all p	onds full and anoth	ner 100yr storm	hits		
Runoff Coeffic	ient Determ	nination							
	Pre-Develop	oment				Post-Develop	Post-Development		
Area	Acres	Coefficient	C*A		Area	Acres	Coefficient	C*A	
Exist. Pervious	2.57	0.35	0.90		Tot Imperv	1.45	0.9	1.	31
			0.00		Proposed Pervic	bus		0.	00
			0.00					0.	00
								0.	00
Total Area	2.57				Undev	1.12	0.35	0.	39
Weighted Avera	ge=	0.35	0.90		Weighted Avera	ge=	0.66	1.	70
0.4005	0.00	0.400.5.4	10.07						
Q 100Pre=	9.89	Q 100 Post=	18.67		1100	11.00	in/nr		
	· · · ·	45.00	Depth of em	nergency of	overflow	0.50	π		
Min. Overflow w	eir length =	15.86	π						



						www.drivene	ngineering.co	<u>m</u>
Dri	ven Fno	nineerina	Inc					
8005 Morris Hill	Road		2143111173					
Sommon AL 2	RUAU 6575			Droject Numb	or		22105	
Semmes, AL 3	0575			Project Numb			23105	
(251)049-4011				Date:			2/20/2024	
	F			Date.			2/29/2024	
Plasmine Car Pa	arking Lot							
Runoff Coeffici	ent Determ	ination						
	Pre-Develop	oment				Post-Develop	oment	
Area	Acres	Coefficient	C*A		Area	Acres	Coefficient	C*A
Exist. imperviou	0.40	0.9	0.36		Pervious	0.35	0.25	0.09
pervious	0.50	0.25	0.13		Proposed	0.55	0.90	0.50
p	0.00	0.20	0.00			0.00	0.00	0.00
			0.00					0.00
Total Area	0.90				Total	0.90		0.00
Weighted Avera	ae=	0.54	0.49		Weighted	Average=	0.32	0.58
Pre and Post F	Jow Data F	or 10. 50 and	100 Year St	orms In CFS:	5	5		Q increase
(Note: Post rund	off values rec	present flow to	detention, no	ot flow from the	e site.)			if not detained
Q 10 Pre=	4.24	Q 10 Post=	5.10		, I10	8.75	in/hr	0.85
Q 25 Pre=	4.41	Q 25 Post=	5.30		125	9.10	in/hr	0.89
Q 50 Pre=	4.90	Q 50 Post=	5.88		150	10.10	in/hr	0.98
Q 100Pre=	5.34	Q 100 Post=	6.41		1100	11.00	in/hr	1.07
Post Developm	ent Contro	Structure to	meet Desig	n release rat	e			
Exist. Rel. Rate	(CFS)=	4.24			Maximu	Im Detention	Volume Calc	ulated Below:
Forced Release	Rate (cfs)=		0.00		619.20	cubic feet		
Release Rate us	sed for calcs	s (cfs)=	4.24					
Pond/Box Invert	=	259.50	Height of we	eir box slot		1.50	ft	
Pond Storage			0					
elevation/								
Popoff=		261.00	Depth of em	ergency overfl	ow	0.30	ft	
Top of berm=		261.50	Freeboard re	equired		0.20	ft	
Req'd Slot width	=	0.69	ft					
Min. Overflow we	eir length =	11.71	ft					
Outflow Rate a	llowed	10	Yr Storm					
Pond Size Reg	uired	50	Yr Storm					
Rainfall	Rainfall	Peak	allowable	release		Storm	Storm	Required
Duration	Intensity	Inflow	release	intensity		Inflow	Outflow	Storage
(Minutes)	(Inches)	(CFS)	(cfs)	(inches)		(Cubic Ft)	(Cubic Ft)	(Cubic Ft)
5	10.1	5.88325	4.24	8.75		1764.98	1273.13	491.85
6	9.9	5.76675	4.24	8.42		2076.03	1527.75	548.28
7	9.7	5.65025	4.24	8.13		2373.11	1782.38	590.73
8	9.5	5.53375	4.24	7.86		2656.20	2037.00	619.20
9	9.2	5.359	4.24	7.61		2893.86	2291.63	602.24
10	9	5.2425	4.24	7.38		3145.50	2546.25	599.25
11	8.65	5.038625	4.24	7.15		3325.49	2800.88	524.62
Detention Pone	d Capacity:							
Description								Volume
			area of	volume incr	total vol to			
		elevation	contour (sf)	to elev (cf)	elev (cf)	length	width	(Cubic Feet)
		259.5	698					
		260	892	397.5	397.5			
	Popoff el	261	1320	1106	1503.5			
	Top Berm	261.5						1503.5
Greatest Require	ed Storage							619.2
						Provided Vol	ume is	ok



## **APPENDIX E: DRAINAGE AREAS MAPS**

## **Pre-Development areas:**





#### **Post-development areas:**



	$\otimes$ = REBAR SET(1/2 AT8 CAFFED MARKED BMS 40196-S flush) $\bigotimes$ = REBAR FOUND(1/2" cap illegible)
251 Newport Parkway Bay Minette AL, 36507	$\square = \text{IRON PIPE FOUND}$ $\square = \text{STEEL POST FOUND}$
Parcel#05-23-02-10-3-000-029.000 PIN 35534	× = SURVEY POINT, NO MONUMENT M = MEASURED DIMENSION
3298 Summit Blvd. Pensacola FL, 32503	R = RECORD DIMENSION AD = ADJACENT DEED DIMENSION
Site Data: Zaging _ City of Pay Minatta M2 (General Inductrial District)	C = CALCULATED DIMENSION 
Site Area -	= APPARENT RIGHT-OF-WAY
Building Setbacks	= PARCEL LINES
Front - 30' Rear - 0'	BOUNDARY LINES
Side -0'	BUILDING SETBACK LINES

#### Plasmine Technology, Inc Instrument#799995

## PARCEL 1

From the Southeast Corner of the Southwest Quarter of Section 10, Township 2 South, Range 3 East, Baldwin County, Alabama; run thence West along South line of said Section 10, 1429 feet to a point; thence North 43 feet to an iron marker, said point being the Southeast Comer of Parcel No. 2, as recorded in Deed Book 52, pages 129-130 of the Probate Record; from said point run thence North 01 °40' East along East line of Newport Industries, Inc., property 216 feet to an iron marker for the Point of Beginning of herein described property; thence continue North 01 °40' East along said property line 915 feet to a point; thence East 258 feet to a point; thence South 01 °40" West, 915 feet to a point; thence West 258 feet to the point of beginning excepting therefrom a triangular portion of 30 foot roadway easement over and across extreme Southeast Comer of above described property.

## PARCEL2

From the Southwest Corner of Section 10, Township 2 South, Range 3 East; run South 89"23'30" East, 63.00 feet; thence run North 00°34'40" East, 100.00 feet; thence run South 89°23'20" East, 867.50 feet to a point; thence run South 00°34'40" East, 57.00 feet to a point; thence run South 89°23'20" East, 275.00 feet to a point; thence run North 00°41 '40" East, 1271.00 feet to a point; thence run South 60°17' West, 162.97 feet to the Point of Beginning; thence continue South 60°17' West, 155.68 feet; thence run North 00°38' East, 133,80 feet to a point on the South right-of-way of the main line of the Louisville & Nashville Railroad, thence run South 67°03' 11" East, 145.22 feet to the Point of Beginning.

Being the same property conveyed to James Graham Brown Foundation, Inc., a Kentucky Corporation, to Reichhold Chemicals, Inc., a Delaware Corporation by deed dated February 27, 1974 and recorded March 1, 1974 in Deed Book 458, page 710, Probate Court Records of Baldwin County, Alabama.

#### PARCEL3

Beginning at a point 63 feet East and 100 feet North of the Southwest corner of Section 10, Township 2 South, Range 3 East; thence due East 867 feet to a point; thence due North 1185 feet to a point on the South line of the right-of-way of the Main Line of the Louisville & Nashville Railroad Co.; thence Westerly and Southerly along the said line of right-of-way of the Main Line of the said Louisville & Nashville Railroad Company and the East line of the right-of-way of the Bay Minette &. Fort Morgan Railroad 1241 feet to the place of beginning.

#### PARCEL 4

Commencing at a point 63 feet East and 100 feet North of the South Corner of Section 10, thence due East 867 feet, which is the Point of Beginning for this parcel; run thence due South 57 feet; thence due East 275 feet; thence due North 1271 feet to a point; thence in a Southwesterly direction and on a line parallel with the South line of the right-of-way of the Main Line of the Louisville & Nashville Railroad Company to a point directly North of the Point of Beginning; thence South 1108 feet along the East line of Parcel No. 3 to the Point of Beginning.

Being the same property conveyed by Tenneco Chemicals, Inc., to Reichhold Chemicals, Inc., by deed dated July 31, 1973 and recorded August 3, 1973 in Deed Book 448, page 971, Probate Court Records of Baldwin County Alabama, LESS AND **EXCEPT THE FOLLOWING:** 

From the Southwest Comer of Section 10, Township 2 South, Range 3 East, run South 89"23 '20" East, 63.00 feet; thence run North 00°34'40" East, 100.00 feet; thence run North 89°23'20" East, 867.50 feet to a point; thence run South 00°34'40" East, 57.00 feet to a point; thence run South 89°23 '20" East, 275.00 feet to a point; thence run North 00°41'40" East, 1131 feet to the Point of Beginning; thence continue North 00°41 '40" East, 140 feet; thence run South 60°17' West, 162.97 feel to a point; thence run South 67°03 '1 l" East, 151.86 feet to the Point of Beginning.

## Also described as:

Commencing at the Southwest comer of Section 10, Township 2 South, Range 3 East; thence run South 89°23'30" East, a distance of 63.00 feet to a point; thence run North 00°34'40" East, a distance of 100 feet to the point of beginning; thence run North 08°04 '58" East, a distance of 120.17 feet to a point; thence Northeasterly along a curve to the right having a radius of 1328.86 feet, a delta angle of 28°41 '33", a chord of which bears North 34°02'03" East, 1084.37 feet, an arc distance of 1116.92 feet to a point; thence run North 61°14'33" East, a distance of 312.73 feet to a point; thence run South 66°06'08" East, a distance of 297.19 feet to a point; thence run South 88°24'54" East, a distance of 258.15 feet to a point; thence run South 01°40'00" West, a distance of 914.90 feet to a point; thence run North 88°21 '10" West, a distance of 257.46 feet to a point thence run South 01°37'26" West, 216.33 feet to a point; thence run North 88°24'44" West, a distance of 275.90 feet to a point; thence run North 01°39'53" East, 57.43 feet to a point: thence run North 88°49'13" West, a distance of 863.69 feet to the point of beginning.

## North Baldwin Utilities

Instrument#1240316

Commence at the Southeast Corner of the Southwest Quarter of Section 10, T2S, R3E, thence run North a distance of 25 feet along the East Boundary of said Southwest Quarter; thence run West a distance of 1,190 feet along a line parallel to and north of the South Boundary of said Southwest Quarter to the POINT OF BEGINNING. Thence run North a distance of 240 feet, more or less, along a line west of and parallel to the East Boundary of said Southwest Quarter to a point; thence run West a distance of 260 feet, more or less, along a line parallel to and north of the South Boundary of said Southwest Quarter to a point; thence run South a distance of 240 feet, more or less, along a line west of and parallel to the East Boundary of said Southwest Quarter to a point; thence run East a distance of 260 feet, more or less, along a line parallel to and north of the South Boundary of said Southwest Quarter to a point, said point being the POINT OF BEGINNING.

Additions or deletions to survey drawings by any other person(s) than the signing party or parties is prohibited without written consent of the signing party or parties.

The Basis of Bearings for this survey is Alabama State Plane Grid (West Zone). The centerlines and fence lines as depicted on this plat are shown graphically as straight lines between random locations as measured. These lines may actually meander between these end points

REVISION DATE PROJECT NO.:1023PLB Plasmine Technology Inc DATED: 11/7/2023 Property Boundary Survey SCALE 1" = 90' 1 Newport Parkway RAWN BY: JRH y Minette, AL 36507 HECKED BY: JRH



**BEYOND MEASURE** SURVEYING 1608 Burtonwood Dr. FOLEY, AL., 36535 (251)-752-7017 W.BEYONDMEASURESURVEYING

Part of the Southwest Quarter of Section Ten (10), Township Two (2) South, Range Three (3) East, Baldwin County, Alabama, more particularly described as follows: Beginning at a steel corner post 63.00 feet South 89 Degrees 23 Minutes 30 Seconds East and 100.00 feet North 00 Degrees 34 Minutes 40 Seconds East of the Southwest corner of said Southwest Quarter; THENCE North 09 Degrees 03 Minutes 26 Seconds East along the Easterly right of way of the CSX railway 121.94 feet to a 1/2" x 18" capped rebar marked BMS 40196S, (hereinafter referred to as a BMS rebar); THENCE continuing along said Easterly right of way and along a curve concave to the Southeast and having an arc length of 684.63 feet and a radius of 1225.12 feet, and being subtended by a chord with a distance of 675.75 feet and a bearing of North 24 Degrees 19 Minutes 48 Seconds East to a BMS rebar; THENCE continuing along said right of way North 60 Degrees 12 Minutes 16 Seconds East 424.50 feet to a BMS rebar; THENCE continuing along said right of way North 29 Degrees 47 Minutes 44 Seconds West 100.00 feet to a BMS rebar; THENCE continuing along said right of way North 60 Degrees 12 Minutes 16 Seconds East 303.21 feet to a steel corner post; THENCE leaving said right of way and running South 67 Degrees 13 Minutes 30 Seconds East 297.35 feet to a 1/2" capped rebar; THENCE South 89 Degrees 29 Minutes 09 Seconds East 258.17 feet to a 1/2" capped rebar; THENCE South 00 Degrees 36 Minutes 35 Seconds West 915.24 feet to a BMS rebar; THENCE North 89 Degrees 24 Minutes 55 Seconds West 257.46 feet to a 1" OD iron pipe; THENCE South 00 Degrees 36 Minutes 14 Seconds West 216.33 feet to a BMS rebar on the Northerly right of way of Newport Parkway; THENCE along said Northerly right of way North 89 Degrees 28 Minutes 30 Seconds West 275.90 feet to a BMS rebar; THENCE North 00 Degrees 28 Minutes 32 Seconds East 56.88 feet to a 1" OD iron pipe; THENCE along said Northerly right of way North 89 Degrees 50 Minutes 38 Seconds West 866.86 feet to the place of beginning, containing 28.731 acres, more or less.

Relative Error of Closure Suburban 1'/7,500' & 20" X sq.root of # of angles

POS -

WA



Together with and subject to all rights of way, covenants, easements and restrictions of record.

## BENCHMARKS:

HORIZONTAL COORDINATES SHOWN ARE ALABAMA STATE PLANE COORDINATES, WEST ZONE, NAD 83

ELEVATIONS SHOWN ARE NAVD88 VERTICAL DATUM.

HORIZONTAL AND VERTICAL DATA WAS DETERMINED BY RTK GPS OBSERVATIONS REFERENCING THE FOLLOWING CORS STATIONS

DM2660	AL92	ALDOT 9 DIV DIS 2 CORS ARP	30d 54m 58.98642
PID	SITE ID	DESIGNATION	LATITUDE
BASE ST	ATIONS USE	D	

	Point Table						
Point #	Elevation	Northing	Easting	Description			
50	264.95	320598.67	1885892.12	CR BMS			
51	259.61	320588.45	1885634.45	CR BMS			
52	260.93	320596.89	1885634.51	CR BMS			
53	256.22	320590.94	1885358.60	CR BMS			
54	266.59	320649.18	1884536.08	CM B409			
55	265.24	320802.17	1885894.21	CR BMS			
504	265.34	320655.02	1885916.31	CPTGPS DEI RED CR			
505	263.68	321015.74	1885926.25	CPTGPS DEI RED CR			

65

LONGITUDE 087d 46m 32.46227s (W)

# PLASMINE TECHNOLOGY 251 NEWPORT PKWY BAY MINETTE, AL 36507 **TRAILER STORAGE & OFF-STREET PARKING DEI PROJECT #23105**



# VICINITY MAP N.T.S.



THIS PROJECT SHALL BE CONSTRUCTED IN ACCORDANCE WITH ALABAMA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION 2022 EDITION.

Malin Fil

AVALISHA L. FISHER, P.E., ALABAMA REG. NO. 22182

03/05/2024 DATE:

TELEPHONE AT&T 702 McMeans Ave. Bay Minette, AL 36507 Business: (251)937-7740

FIBER BALDWIN COUNTY COMMISSION Shannon Spivey 111 Blackburn Ave Bay Minette, AL 36507 Business: (251)580-5055 251.937.9561 - North Baldwin County 251.943.5061 - South Baldwin County citizenservicecenter@baldwincountyal.gov FIBER

MCI TELECOMMUNICATIONS

906 McMeans Ave

Bay Minette, AL 36507

Business: (251)937-2828



PLANS NOT VALID UNLESS THEY BEAR A COLOR SIGNATURE OR AN EMBOSSED SEAL PLANS ARE NOT ISSUED FOR CONSTRUCTION UNLESS THE REVISION IS A NUMERAL.

POWER

ALABAMA POWER

150 St. Joseph Street

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23105

10

RAWING NUMBER TOTAL SHEETS

AS SHOWN

REVISIO

03/04/2024

ROJECT NUMBER:

C1.0

David Richardson

Mobile, AL 36602

GENERAL PROJECT NOTES

- 1. STANDARD SPECIFICATIONS FOR STREETS AND DRAINAGE: REFERENCE IS MADE TO THE ALABAMA DEPARTMENT OF TRANSPORTATION "STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION", 2022 EDITION. ALL PROVISIONS OF SAID STANDARD SPECIFICATIONS SHALL APPLY TO THIS CONTRACT AND ARE HEREBY MADE A PART OF THIS CONTRACT, EXCEPT WHEN THE PROVISIONS HEREON OR THE PLANS ARE CLEARLY IN CONFLICT WITH THE PROVISIONS OF SAID STANDARD SPECIFICATIONS, THE PROVISIONS HEREON AND THE PLANS SHALL GOVERN.
- 2. THE CONTRACTOR IS TO FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, AND CONDITIONS PRIOR TO CONSTRUCTION OR FABRICATION.
- 3. THESE DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND ANY APPLICABLE SPECIFICATIONS PROVIDED FOR THIS PROJECT. CONTRACTOR IS DIRECTED TO NOTIFY THE ENGINEER OF RECORD IMMEDIATELY IF ANY CONFLICT IS FOUND IN THESE PLANS AND/OR BETWEEN THESE PLANS OF OTHER DISCIPLINES FOR THIS PROJECT.
- 4. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS EVEN THOUGH NOT SPECIFICALLY CALLED FOR ON THE DRAWINGS.
- 5. ALL UNPAVED AREAS THAT HAVE BEEN GRADED, CUT, OR FILLED SHALL BE TREATED WITH A SUITABLE COMMERCIAL FERTILIZER IN ACCORDANCE WITH ALABAMA DEPARTMENT OF TRANSPORTATION 2022 STANDARD SPECIFICATIONS, AND SEEDED WITH A MIXTURE TO SUIT THE PLANTING ZONE (652.03) AND DATE OF PLANTING (860.01) PER ALABAMA DEPARTMENT OF TRANSPORTATION 2022 STANDARD SPECIFICATIONS. A FIRM STAND OF PERMANENT GRASS WILL BE REQUIRED.
- 6. ALL CONCRETE USED ON THE PROJECT SHALL BE 4,000 PSI MINIMUM COMPRESSIVE STRENGTH REQUIRED IN 28 DAYS, UNLESS SPECIFICATIONS REQUIRE CONCRETE OF GREATER STRENGTH.
- 7. UNDERGROUND UTILITY LOCATIONS SHOWN ARE APPROXIMATE AND ARE BASED ON INFORMATION PROVIDED. THE UTILITIES SHOWN MAY NOT BE A COMPLETE REPRESENTATION OF ALL UTILITY LINES IN THE PROJECT AREA. CONTRACTOR IS REQUIRED TO CONTACT ALABAMA ONE CALL PRIOR TO DIGGING (1-800-292-8525) (WWW.AL1CALL.COM). OTHER UTILITIES (INCLUDING PRIVATE UTILITIES INSIDE OR OUTSIDE A PUBLIC RIGHT-OF-WAY). THAT DO NOT PARTICIPATE IN THE ALABAMA ONE CALL LINE LOCATION SERVICE NEED TO BE CONTACTED INDIVIDUALLY AND/OR PHYSICALLY LOCATED BY THE CONTRACTOR.
- 8. ALL ROUND REINFORCED CONCRETE PIPE USED WITHIN THE CITY, COUNTY OR STATE RIGHT-OF-WAY MUST BE CLASS 3 WITH RUBBER GASKETS. ALL ARCH REINFORCED CONCRETE PIPE USED WITHIN COUNTY OR STATE RIGHT-OF-WAY MUST BE CLASS 3 WITH RAM-NEK MASTIC GASKETS.
- 9. THE WORD OF THE ENGINEER AND/OR INSPECTOR SHALL BE FINAL IN ANY MATTER.
- 10. SUB-GRADE AND BASE SHALL BE COMPACTED TO THE REQUIREMENTS OF ALABAMA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED.
- 11. THE CONTRACTOR WILL BE RESPONSIBLE FOR PROPER COMPACTION ON ANY AND ALL UTILITY DITCHES INCLUDING TAMPING WITH HANDHELD COMPACTORS.
- 12. ALL FILL AND EMBANKMENT CONSTRUCTION SHALL BE COMPACTED AS REQUIRED IN LAYERS NOT TO EXCEED 8".
- 13. ALL SUITABLE EXCESS UNCLASSIFIED EXCAVATION IS TO BE UTILIZED FOR CONSTRUCTION OF EMBANKMENTS AND SLOPES NOT DIRECTLY UNDER THE TRAVEL WAY OR PARKING AREAS PRIOR TO USING ANY OFFSITE BORROW EXCAVATION. AFTER CONSTRUCTION OF SUCH AREAS IS COMPLETED, EXCESS EXCAVATION SHALL BE HAULED FROM THE SITE AT NO ADDITIONAL PAYMENT.
- 14. ALL SEDIMENT CONTROL DEVICES SHALL BE CONSTRUCTED AND FULLY FUNCTIONING PRIOR TO ANY OTHER CONSTRUCTION OR GRADING ACTIVITY.
- 15. ALL SLOPES MUST BE STABILIZED AS SOON AS POSSIBLE TO PREVENT EXCESSIVE EROSION.
- 16. THE SITE IS LOCATED IN SEC. 10, TOWNSHIP 2S, RANGE 3E. BALDWIN COUNTY, (ALABAMA).
- 17. PRELIMINARY SOILS TESTING AND ON-SITE CONSTRUCTION MATERIALS TESTING IS (TO BE) PERFORMED BY AN INDEPENDENT GEOTECHNICAL ENGINEER. THE GEOTECHNICAL ENGINEER FOR THIS PROJECT IS TO BE DETERMINED BY OWNER.
- 18. ALL MATERIALS SHALL BE NEW UNLESS USED OR SALVAGED MATERIALS ARE AUTHORIZED BY THE OWNER.
- 19. HIGH INTENSITY LIGHTING FACILITIES SHALL BE SO ARRANGED THAT THE SOURCE OF ANY LIGHT IS CONCEALED FROM PUBLIC VIEW AND FROM ADJACENT RESIDENTIAL PROPERTY AND DOES NOT INTERFERE WITH TRAFFIC.
- 20. CONTRACTOR IS PROHIBITED FROM DISTURBING SITE AREAS OUTSIDE THE CONSTRUCTION LIMITS SHOWN ON THE PLANS WITHOUT PRIOR APPROVAL FROM THE ENGINEER. STAGING AREAS OUTSIDE THE CONSTRUCTION LIMITS, PILES OF DIRT, AND OTHER BARE AREAS ARE TO BE COVERED AS DIRECTED BY THE ENGINEER. ANY AREAS DISTURBED OUTSIDE THE CONSTRUCTION LIMITS WILL BE REPAIRED AND COVERED WITH A FIRM STAND OF GRASS BEFORE FINAL PAYMENT AND FINAL ACCEPTANCE OF THE PROJECT AT NO ADDITIONAL COST TO THE PROJECT OWNER. IF GRASS WILL NOT GROW ON THE SUBJECT AREA DUE TO POOR WEATHER CONDITIONS, THE CONTRACTOR AGREES TO PLACE SOD ON THE AREA TO PREVENT EXCESS BARE AREAS FROM CONTRIBUTING SEDIMENT EROSION TO THE AREAS OF THE SITE THAT ARE UNDER CONSTRUCTION.
- 21. CONTRACTOR IS REQUIRED TO USE "BEST MANAGEMENT PRACTICES" COMPLIANT WITH THE "ALABAMA HANDBOOK FOR EROSION CONTROL AND STORMWATER MANAGEMENT ON CONSTRUCTION SITES AND URBAN AREAS", ALABAMA SOIL AND WATER CONSERVATION COMMITTEE, MONTGOMERY, ALABAMA, VOLUMES 1 & 2, 2018 EDITION, TO PREVENT SEDIMENT LADEN STORM WATER RUNOFF OR ERODED MATERIALS FROM LEAVING THE CONSTRUCTION SITE.
- 22. ALL MATERIALS AND WORKMANSHIP WITHIN A STATE OR COUNTY RIGHT-OF-WAY SHALL CONFORM TO THE ALABAMA DEPARTMENT OF TRANSPORTATION SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, 2022 EDITION.
- 23. IF MORE THAN 1 ACRE WILL BE DISTURBED IN TOTAL FOR CONSTRUCTION OF THIS SITE, THE CONTRACTOR IS REQUIRED TO OBTAIN AN ADEM CONSTRUCTION STORMWATER (NPDES) PERMIT PRIOR TO CONSTRUCTION. CONTRACTOR IS REQUIRED TO RETAIN A QUALIFIED CREDENTIALED PROFESSIONAL (QCP) TO APPLY FOR THE PERMIT, PREPARE A CONSTRUCTION BEST MANAGEMENT PRACTICES PLAN (CBMPP) DOCUMENT, AND TO COMPLETE THE REQUIRED INSPECTIONS, REPORTING, AND PERMIT TERMINATION. DRIVEN ENGINEERING HAS PREPARED A BEST MANAGEMENT PRACTICES (BMP) PLAN (NOT TO BE CONFUSED WITH THE CBMPP DOCUMENT) AS PART OF THESE DETAILED PLANS. ANY OTHER THAN ONE EMPLOYED BY DRIVEN ENGINEERING WILL NEED TO GENERATE THEIR OWN BMP PLAN DESIGN AS PART OF THEIR CBMPP. THE CONTRACTOR MUST HAVE A RAIN GAUGE INSTALLED ON SITE IN A LOCATION AT LEAST 50' AWAY FROM ALL BUILDINGS, TREES, AND OTHER VERTICAL OBSTRUCTIONS. EVERY 24 HOURS THE RAIN GAUGE READING MUST BE RECORDED AND KEPT WITHIN THE CBMPP PLAN ON SITE (GENERALLY AT THE SAME HOUR OF EACH DAY) AND THE SELECTED QCP MUST BE NOTIFIED IMMEDIATELY OF EVERY RAIN EVENT THAT EXCEEDS 0.75 INCHES IN 24 HOURS. CONTRACTOR MUST MAINTAIN RECORDS OF DAILY "SELF-INSPECTIONS" OF THE SITE IN ADDITION TO THE QCP ACTIVITIES, USING THE CBMPP PLAN FOR THE PROJECT.
- 24. ONCE THE CONSTRUCTION IS COMPLETED AND ACCEPTED BY THE OWNER, MAINTENANCE OF THE STORMWATER DETENTION FACILITY IS THE RESPONSIBILITY OF THE PROPERTY OWNER.
- 25. REQUIRED STORMWATER DETENTION FACILITY MAINTENANCE ACTIVITIES WILL INCLUDE PERIODIC REMOVAL OF SILT AND PERIODIC REMOVAL OF WEEDS AND OTHER DEBRIS.
- 26. COST OF MAINTAINING THE STORMWATER DETENTION FACILITY IS ESTIMATED TO BE APPROXIMATELY \$5000 PER YEAR.
- 27. A SAFETY FENCE SURROUNDING THE DETENTION POND IS RECOMMENDED.
- 28. THE TREES INDICATED ARE TO BE PRESERVED. THE CONTRACTOR IS DIRECTED TO EXERCISE EXTREME CAUTION WHEN WORKING NEAR THESE TREES. NO VEHICLES OR HEAVY EQUIPMENT SHALL BE PARKED OR STORED UNDER THE CANOPY OF THESE TREES. WHEN SUBGRADE IS EXCAVATED IN THE ROOT ZONE OF THESE TREES, ROOTS ARE TO BE SAWED NOT RIPPED WITH HEAVY EQUIPMENT SUCH AS BACKHOE BUCKETS. NO METHOD OF ROOT REMOVAL WILL BE DONE BY ANY METHOD THAT WOULD RESULT IN ANY DISPLACEMENT OF ROOTS THAT ARE TO REMAIN.

# LEGEND

# PATTERNS

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PROPOSED ASPHALT	XX.X
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## SYMBOLS

LIGHT POLE
GUY POLE W/ANCHOR
ELECTRIC SERVICE POLE
ELECTRIC VAULT
FIRE HYDRANT
STOP SIGN
OAK TREE
PROPOSED SPOT ELEVATION
EXISTING SPOT ELEVATION
PROPOSED LAYOUT POINT
WATER VALVE
CURB STOP
TREE TO BE REMOVED (ALL TYPES)
CONCRETE MONUMENT
CONTROL POINT
CAPPED REBAR
CRIMP TOP PIPE
GAS METER
GUY WIRE
MAIL BOX
MAN HOLE
MARKER
GAS MARKER
WATER MARKER
COMMUNICATION MARKER
POWER POLE
STREET SIGN
TELEPHONE PEDESTAL
WATER METER
SINGLE TREE REMOVAL
MULTIPLE TREE REMOVAL
FH VALVE
ELECTRICAL SERVICE BOX
GAS VALVE
GAS METER
GRATE INLET
DOUBLE INLET SINGLE INLET LEFT SINGLE INLET RIGHT
BOX INLET W/ MANHOLE
ELECTRICAL MANHOLE
TELEPHONE MANHOLE
CABLE TV PEDESTAL
SOIL BORING

TREE TO BE PROTECTED & PRESERVED

LINE	IPES
101	PROPOSED INT
100	PROPOSED INI
	PROPOSED PR
	PROPOSED FEI
0 0 0	PROPOSED CH
	PROPOSED ED
	PROPUSED SE
	PROPOSED WA
6"W	PROPOSED 8"
4"M	PROPOSED 6
2.5"WL	PRUPUSED 4
2"WL	PROPOSED 2"
	PROPOSED 1"
SS	PROPOSED SA
11	EXISTING INTER
	EXISTING INDE
artege Good Heigerge Good Heigerin. 🔽 Generalizenin, fait antegen, fait ante	EVISTING FASE
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EXISTING WROU EXISTING TREE EXISTING OVE

TERMEDIATE CONTOUR
DEX CONTOUR
OPERTY LINE
NCE (TYPE TBD BY OWNER)
IAIN LINK (ALUMINUM) FENCE
GE OF PAVEMENT
PARATION FABRIC
ATER LINE
WATER LINE
WATER LINE
WALER LINE
WATER LINE
WATER LINE
NITARY SEWER LINE
RMEDIATE CONTOUR
X CONTOUR
EMENT
PERTY LINE
IT-OF-WAY
E OF PAVEMENT
RHEAD UTILITY LINE(S)
ERGROUND TELEPHONE LINE
ERGROUND GAS LINE OR PIPELINE
TARY SEWER LINE
FR LINE
RD RAIL LEFT
RAIL RIGHT
Y SEWER STEEL ENCASEMENT LINE (BY OTHERS)
ED LINE
Y ТВА
4
ROPERTY LINES
NSTRUCTION
ENCE
ELEVATION LINE
PIPE
D PRIVACY FENCE
N LINK FENCE
UGHT FENCE
LINE
RHEAD CABLE

AE	BREVIATIONS
(A)	ACTUAL
(R)	RECORD
(M)	FIELD MEASUREMENT
(P)	PLAT (UNRECORDED)
B.O.B.	BASIS OF BEARINGS
P.O.B.	POINT OF BEGINNING
P.O.C.	POINT OF COMMENCEMENT
R/W	RIGHT-OF-WAY
тва	TO BE ACQUIRED
FFF	FINISHED FLOOR FLEVATION
USFPE	CORRUGATED SMOOTH FLOW PULTETHTLENE PIPE
LS	LUMP SUM
SY	SQUARE YARD
LF	LINEAR FOOT
EA	EACH
CY	CUBIC YARD
STA	STATION
GAL	GALLON
AC	ACRE
SF	SQUARE FOOT
LB	POUND
REQD	REQUIRED
FOP	FDGE OF PAVEMENT OR END OR PROJECT
FX/FXIST	FXISTING
MIN	MINIMUM
ROP	BEGINNING OF PROJECT
TYP	
REI	
FU	FIBER OPTIC
ሢ	CENTER LINE
0.C.	ON CENTER
WL	WATER LINE
BFE	BASE FLOOD ELEVATION
TBM	TEMPORARY BENCH MARK
MAINT.	MAINTENANCE
DEMO(D)	DEMOLITION/DEMOLISHED
TBD	TO BE DETERMINED
R.O.W.	RIGHT OF WAY
R/W	RIGHT OF WAY
Ô	AT
SYMM	SYMMETRICAL
UMU	CONGRETE MASONRT UNIT
PSI	POUNDS PER SQUARE INCH
CRF	CAPPED REBAR FOUND (BUSBY)
CMF	4"x4" CONCRETE MONUMENT FOUND
CTF	CRIMP TOP PIPE FOUND
RBF	5/8" REBAR FOUND
CRS	CAPPED REBAR SET
VCP	VITRIFIED CLAY PIPE
D.I.	
U.I. СПТ	UAST IKUN Sufet
CIPP	CLIRED IN PLACE PIPE
VII I	JUNED IN LEADE FILE



PLANS NOT VALID UNLESS THEY BEAR A COLOR SIGNATURE OR AN EMBOSSED SEAL PLANS ARE NOT ISSUED FOR CONSTRUCTION UNLESS THE REVISION IS A NUMERAL.



	Layout	Point Tab	ole
Point #	Northing	Easting	Description
100	320797.41	1885621.65	EOC
101	320805.54	1885648.47	EOC
102	320871.73	1885743.01	EOC
103	320870.29	1885886.24	EOC
104	320744.24	1885889.15	EOC
105	320781.12	1885818.33	EOC
106	320685.61	1885681.93	EOC
107	320671.69	1885636.98	EOC
108	320672.01	1885605.24	EOC
109	320662.73	1885602.60	EOC
110	320624.58	1885665.68	EOC
111	320626.28	1885498.63	EOC
112	320692.34	1885376.79	EOC
113	320685.56	1885400.64	EOC
114	320692.58	1885353.84	EOC
115	320680.17	1885322.35	EOC
116	320760.80	1885360.25	EOC
117	320795.72	1885485.37	EOC

	Layout	Point Tab	ble
Point #	Northing	Easting	Description
118	320798.55	1885506.83	EOC
119	320788.26	1885378.58	EOP
120	320812.35	1885366.64	EOP
121	320817.35	1885483.11	EOP
122	320817.67	1885494.84	EOP
123	320830.21	1885478.94	EOP
124	320836.78	1885483.74	EOP
125	320836.71	1885490.03	EOP
126	320832.14	1885494.94	EOP
127	320876.42	1885467.18	EOP
128	320879.84	1885477.46	EOP
129	320864.62	1885477.31	EOP
130	320863.73	1885470.95	EOP
131	320833.22	1885540.00	EOP
132	320836.19	1885543.03	EOP
133	320836.16	1885546.03	EOP
134	320860.19	1885543.27	EOP
135	320860.16	1885546.27	EOP
136	320833.13	1885549.00	EOP
137	320817.22	1885539.84	EOP

	Layout	Point Tab	ble
Point #	Northing	Easting	Description
138	320817.13	1885548.84	EOP
139	320879.22	1885540.46	EOP
140	320879.13	1885549.46	EOP
141	320863.22	1885540.30	EOP
142	320863.13	1885549.30	EOP
143	320878.24	1885639.45	EOP
144	320864.24	1885639.32	EOP
145	320859.14	1885649.27	EOP
146	320835.14	1885649.02	EOP
147	320830.24	1885638.98	EOP
148	320816.24	1885638.84	EOP
149	320863.24	1885664.31	PND
150	320863.24	1885704.73	PND
151	320834.74	1885664.03	PND
152	320905.56	1885609.35	PND
153	320930.54	1885609.54	PND
154	320928.28	1885838.14	PND
155	320903.28	1885837.90	PND
156	320759.96	1885831.70	PND
157	320683.23	1885828.33	PND

	Layout	Point Tab	ble
Point #	Northing	Easting	Description
158	320648.51	1885655.10	PND
159	320654.49	1885674.19	PND
160	320607.88	1885634.71	FNC
161	320607.88	1885830.08	FNC
162	320653.57	1885845.98	FNC
163	320729.26	1885384.87	PRK
164	320761.38	1885502.41	PRK
165	320731.81	1885468.89	PRK
166	320723.67	1885442.62	PRK
167	320724.19	1885686.11	PRK
168	320729.32	1885712.97	PRK
169	320758.60	1885754.80	PRK
170	320755.93	1885726.22	PRK
171	320761.06	1885753.08	PRK
172	320798.66	1885792.48	PRK
173	320837.85	1885850.53	PRK
174	320833.19	1885856.09	PRK
175	320803.79	1885819.34	PRK

PROPERTY IS LOCATED IN AN "X" UNSHADED FLOOD ZONE. MAP 01003C0294M, DATED 04/19/2019

Plasmine Technology, Inc. Instr#799995, 4/5/2004 28.731 Acres(measured)

REQ'D ± 1085 SY SOD OR

SEEDED AREA. ALSO

REQ'D BIG TRUCK

a wind

(TYP)

REQ'D ASPHALT CONSTRUCTION JOINT SEE DETAIL SHEET C7.0

EXISTING ROW LINE

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and the first what

PARKING SIGN

REQ'D ± 2521 SF THERMOPLASTIC

PAINTED ISLAND

(SEE DETAIL

R45

SHEET C7.0)

R229'

SG S

(RELOCATED)

ARKING

AVAILABLE FOR

LANDSCAPING

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	REQ'D PIPES		
NAME	DESCRIPTION	LENGTH	SLOPE
P-1	24 " RCP	108'	0.37%
P-2	18" RCP	180'	0.92%
P-3	15" N12 HDPE	8'	0.63%
P-4	15" N12 HDPE	17'	0.58%
P-5	24" RCP	81'	0.52%
P-6	24" N12 HDPE	11'	0.94%



		REQ'D STRUCT	URES		
NAME	DESCRIPTION	PIPES IN	PIPES OUT	RIM	LAYOUT COORDINATES
HVV-1	24" PIPE END TREATMENT	PIPE "P-5", INV IN =257.43		259.7	N=320599.2 E=1885572.1
HW-2	15" PIPE END TREATMENT		PIPE "P-4",INV OUT=258.00	259.0	N=320648.6 E=1885656.0
HW-3	24" PIPE END TREATMENT	PIPE "P-1", INV IN =258.00		260.2	N=320772.2 E=1885831.6
HW-4	24" PIPE END TREATMENT		PIPE "P-6",INV OUT=258.50	260.8	N=320890.8 E=1885831.3
JB-1	JUNCTION BOX	PIPE "P-2", INV IN =257.85 PIPE "P-3", INV IN =257.85	PIPE "P-5",INV OUT=257.85	262.2	N=320654.2 E=1885631.3
VB-2	JUNCTION BOX WITH MOUNTED GATE VALVE	PIPE "P-4", INV IN =257.90	PIPE "P-3",INV OUT=257.90	261.8	N=320652.8 E=1885639.2
VB-3	JUNCTION BOX WITH MOUNTED GATE VALVE	PIPE "P-6", INV IN =258.40	PIPE "P-1",INV OUT=258.40	263.2	N=320880.2 E=1885831.4
WS-1	POND OUTFALL STRUCTURE		PIPE "P-2",INV OUT=259.50	262.9	N=320831.1 E=1885663.1

CONTAINED CONCRETE TRUCK PARKING AREA (PER SPCC SPECIFICATIONS) = 1.6 AC NORTH POND = 0.43 AC SOUTH POND = 0.35 AC

25 YR 24 HR RAINFALL = 10 INCHES 100 YR 24 HR RAINFALL = 12 INCHES

POND VOLUME REQ'D (25 YR) (2.57 AC) X (43,560 SF) X (0.83 FT) = 92,900 CF

POND VOLUME REQ'D (100 YR) (2.57 AC) X (43,560 SF) X (1 FT) = 112,000 CF

AT EMERGENCY OVERFLOW ELEVATION 262.25: NORTH POND VOLUME = 54,851 CF SOUTH POND VOLUME = 61,114 CF TOTAL VOLUME = 115,965 CF

CONTACT ENGINEER FOR ANY ADDITIONAL POINT **INFORMATION AND/OR 3D CAD FILES.** 



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# PRE-CAST CONCRETE PARKING STOP

- BE 12 SQ IN WITH A MAXIMUM DIMENSION OF 2 INCHES IN ANY ONE DIRECTION.
- NOT LESS THAN #9 WIRE STAPLES 1.5 INCHES LONG.
- 4. BURY GEOGRID A MINIMUM OF 6" BELOW GROUND

![](_page_19_Figure_10.jpeg)

TREE PROTECTION DETAIL

N.T.S

![](_page_19_Figure_15.jpeg)

♦4 REBARS ➡—#4 × 1'−6" (457 mm) REBAR

![](_page_19_Picture_17.jpeg)

![](_page_19_Figure_18.jpeg)

RECYCLED CONCRETE BASE WILL BE CONSIDERED AS AN APPROVED EQUAL TO CRUSHED AGGREGATE BASE COURSE.

(7) EXISTING SOILS

6 230-A IMPROVED ROADBED (100% STANDARD DENSITY) 6"

5 825-A CRUSHED AGGREGATE BASE COURSE, TYPE B. 6" THICK (100% MODIFIED DENSITY)

401-A BITUMINOUS TREATMENT, TYPE A

(3) 429-B, IMPROVED BITUMINOUS CONCRETE UPPER BINDER LAYER, 1" MAX. AGGREGATE SIZE MIX, ESAL RANGE B, 165#/SY

2 405-A TACK COAT

BARBED-WIRE APRON -ON EXTENSION ARMS

INDEX	ALABAMA DEPARTMENT OF STANDARD HIGHWAY DRAV DWG. NO.	F TRANSPORTATION SPECIAL AND WING REFERENCES, 2022 EDITION: DESCRIPTION
53006	RPC-530	BEDDING AND FILL HEIGHTS FOR ALL ROADWAY PIPE CULVERTS
		(H.D.P.E. PIPE)
61909 - 61910	HW-614-B	SLOPE PAVED HEADWALL DETAILS FOR REINFORCED CONCRETE AND CORRUGATED METAL ROADWAY PIPE
61913	HW-614-SP	CONCRETE SLOPE PAVED HEADWALL AND GRATE FOR SIDEDRAIN PIPE
66512	ESC-300-1	DITCH CHECK STRUCTURES, TYPICAL APPLICATIONS AND DETAILS
66515	ESC-300-4	DETAILS OF EROSION CONTROL WATTLE DITCH CHECKS
66517	ESC-300-6	DETAILS OF ROCK DITCH CHECKS
66518	ESC-300-7	DETAILS OF ROCK DITCH CHECKS WITH SUMP EXCAVATION
66519	ESC-300-8	DETAILS OF SILT FENCE DITCH CHECKS
66522	ESC-400-1	INLET PROTECTION TYPICAL APPLICATIONS AND DETAILS
66523	ESC-400-2	INLET PROTECTION DETAILS FOR COARSE AGGREGATE ON GRADES AND SAGS
66524	ESC-400-3	INLET PROTECTION DETAILS OF WATTLES
66525	ESC-400-4	INLET PROTECTION DETAILS OF SILT FENCE
66532	ESC-502	STABILIZED CONSTRUCTION ENTRANCE

![](_page_20_Figure_2.jpeg)

![](_page_20_Figure_3.jpeg)

![](_page_20_Picture_4.jpeg)

![](_page_20_Figure_5.jpeg)

![](_page_21_Picture_0.jpeg)

![](_page_22_Figure_0.jpeg)

	А	SITE PL	AN SUBMITTA	L
AFASTIDES SHOWN ADE CONSIDEDED TO DE THE MINIMUM				
TRACTOR SHALL LITILIZE "REST MANAGEMENT PRACTICES" AS				
LADEN STORMWATER RUNOFF OR ERODED MATERIALS FROM LEAVING				
NTRACTOR SHALL MAINTAIN AND REPAIR EROSION CONTROL				
NER AFTER EACH RAINFALL EVENT AND INSPECT THEM TWICE			Ĩ	Т
IFALL, BEST MANAGEMENT PRACTICES (BMPS) ARE DEFINED AS:			.0-	
IT OR REDUCE THE POLLUTION OF WATERS OF THE UNITED STATES.			1	
EQUIREMENTS, OPERATING PROCEDURES, AND PRACTICES TO		•		_
LAGE OR LEAKS, SLUDGE OR WASTE DISPOSAL, OR DRAINAGE FROM		-		
ARD TO CONSTRUCTION THESE MAY INCLUDE STRUCTURAL DEVICES				23
ALANE DESIGNED TO FILTENT FOLLUTANTS FROM ENTERING WATER	11/	C I TTH 17C Ø	CDEENING	<b>a</b>

W TECHN THES VISIBLI COPIED QUALI SURE QUALI C ENGINI TI	E UTILIZ IQUES IN SE SOME 3 IF OUR OR PRID TY SETT THAT YO TY PRIN CAN SEE DERING, 1 HE TEXT	E SCREE I OUR DE TIMES A DESIGN NTED WI ING. YO OU HAVI I OR COI THE DRI INC. LOG IN THIS	NING SIGN AND RE NOT HAS BEEN TH A LOW U CAN BE 3 A HIGH PY IF YOU VEN KO BEHIND BOX.
PLASMINE TECHNOLOGY, INC	251 NEWPORT PKWY	<b>BAY MINETTE, AL 36507</b>	
	PROFE	ENSE ENSE 2182 ESSIONAL	
Driven		Engineering, Inc.	BUUD Morris Hill Koad, Semmes, AL 365/5 (251) 649-4011 Office (251) 645-0971 Fax www.drivenengineering.com
PLASMINE	TRAILER STORAGE & OFF-STREET	PARKING CONSTRUCTION PLANS	EROSION CONTROL DETAILS

PROJECT NUMBER:

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DRAWING NUMBER TOTAL SHEETS