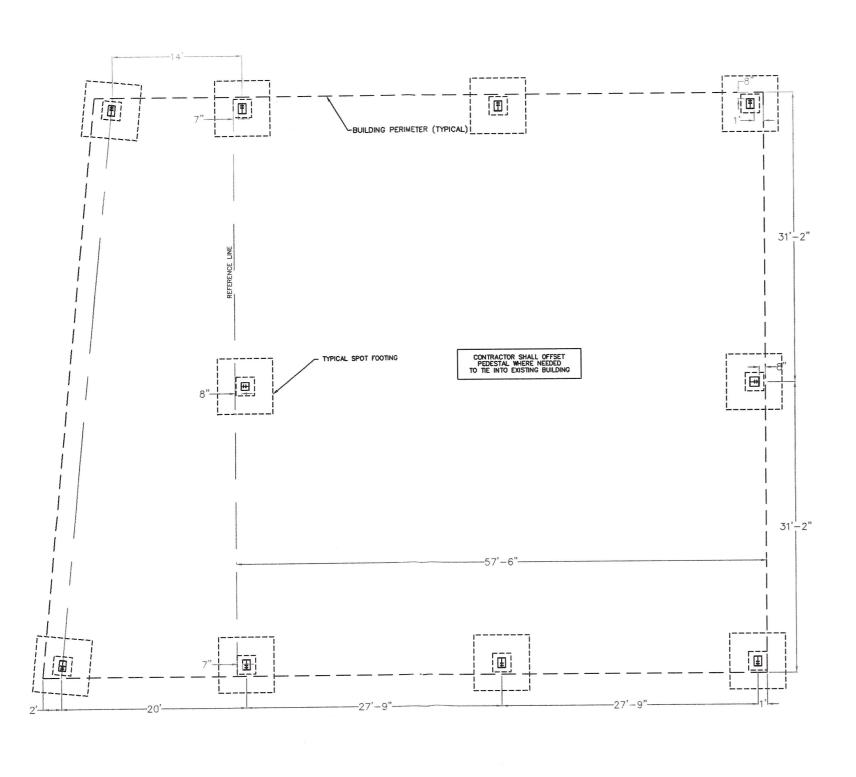
Exhibit B SP-24003, Stella-Jones Property Site Plan Request

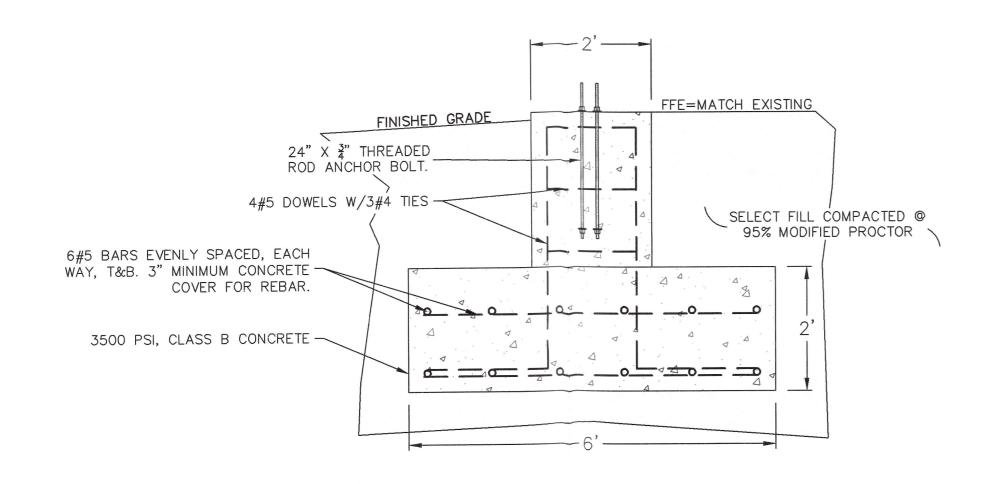
Foundation and Building Plans

Sheet C 1

Project No. 19-024



FOUNDATION PLAN SCALE:1"=10'



TYPICAL SPOT FOOTING SCALE:N/A



CONCRETE NOTES:

1. MINIMUM CONCRETE REINFORCING COVER REQUIREMENTS:

CAST IN PLACE
CONCRETE COVER:

A. CONCRETE CAST AGAINST EARTH = 3"

B. FORMED CONCRETE EXPOSED TO EARTH

OR WEATHER:
WALLS PANELS AND SLABS:
#5 BAR AND LARGER = 2"
#5 BAR AND SMALLER = 1½"
OTHER MEMBERS:
#5 BAR AND LARGER = 2"
#5 BAR AND SMALLER = 1½"
C. BEAMS AND COLUMNS:
PRIMARY REINFORCEMENT = 1½"

TIES, STIRRUPS AND SPIRALS = 1 ½"

2. MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS WILL BE AS FOLLOWS, UNLESS NOTED OTHERWISE: FOUNDATIONS, FOOTINGS, RETAINING WALLS, SLAB ON GRADE AND ALL OTHER CONCRETE = 3500 psi (CLASS "B")

3. ALL CONDUIT, SLEEVES AND PIPES EMBEDDED IN CONCRETE SHALL CONFORM TO SECTION 6.3 OF ACI 318 AND THE FOLLOWING:

A CONCRETE REAMS SLAB RIBS AND JOIST WIDTHS SHOWN ON THE DRAWINGS ARE MINIMUM ALLOWABLE

A. CONCRETE BEAMS, SLAB RIBS AND JOIST WIDTHS SHOWN ON THE DRAWINGS ARE MINIMUM ALLOWABLE WIDTHS.

B. BEAMS, SLAB RIBS, AND JOISTS HAVING PIPES OR SLEEVES PASSING THROUGH THEM WHICH ARE NOT SHOWN

ON THE STRUCTURAL DRAWINGS SHALL BE INCREASED IN WIDTH IMMEDIATELY ADJACENT TO THE SLEEVES OR PIPE TO OBTAIN THE SAME CROSS SECTIONAL AREA OF CONCRETE SHOWN FOR THE MEMBER.

C. SLEEVES AND PIPES SHALL BE PLACED SO THAT REINFORCING STEEL CAN BE PLACED WITH THE SPECIFIED

COVER AND CLEAR DISTANCE BETWEEN BARS.

D. THE CONCRETE COVERING OF PIPE AND SLEEVES SHALL NOT BE LESS THAN ONE INCH. CLEAR DISTANCE BETWEEN SUCH PIPES AND SLEEVES SHALL NOT BE LESS THAN ONE HALF INCHES.

E. CONDUIT AND PIPES PLACED IN SLABS AND TOPPING OVER SLAB RIBS OR JOISTS SHALL NOT BE LARGER IN OUTSIDE DIAMETER THAN ONE—THIRD THE THICKNESS OF SLAB OR TOPPING. NOT MORE THAN 1 ¼ INCH ROUND O.D. CONDUIT OR THE EQUIVILENT AREA IN SMALLER CONDUIT SHALL BE PLACED IN ANY SIX INCH WIDE JOIST OR SLAB RIB. IF IT IS NECESSARY TO USE LARGER CONDUIT OR PIPES, THE SLAB OR TOPPING SHALL BE THICKENED, SLAB OR JOIST RIBS SHALL BE WIDENED AND REINFORCING ADDED TO SUPPRT THE ADDITIONAL WEIGHT OF THE CONCRETE.

ADDITIONAL WEIGHT OF THE CONCRETE.

F. CONDUITS OR PIPES PASSING THROUGH JOISTS, SLAB RIBS OR BEAMS PARRALEL TO THE MEMBER SHALL BE NOT LARGER THAN 1 ¼ INCH DIAMETER O.D. AND SHALL BE PLACED 2" CLEAR OF REINFORCING AT TOP, BOTTOM AND SIDES. CODUITS OR PIPES PASSING THROUGH JOISTS, SLAB RIBS OR BEAMS PERPENDICULAR TO THE MEMBER SHALL NOT BE LARGER THAN 2" O.D. AND SHALL BE PLACED AT MID—HEIGHT OF THE MEMBER.

G. CONDUITS AND PIPES PLACED IN COLUMNS SHALL NOT DISPLACE MORE THAN 4% OF THE CROSS SECTIONAL

AREA OF THE COLUMN AND SHALL BE LOCATED ON THE CENTER LINE OF THE COLUMN. ELECTRICAL BOXES

SHALL BE NO DEEPER THAN REQUIRED CLEARANCE FOR REINFORCING.

6. NO HORIZONTAL CONSTRUCTION JOINTS WILL BE ALLOWED IN GRADE BEAMS.

November 14, 2024

DESIGN LOADING

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED BY THE

IBC 2021

IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

SPECIFIC LOADS: SEE STRUCTURAL CALCULATIONS AND FOUNDATION REACTIONS.



20.00 ROOF LIVE LOAD (psf)

Yes LIVE LOAD REDUCTION ALLOWED?

1.00 COLLATERAL LOAD (psf)

0.0 GROUND SNOW LOAD, Pg (psf)

145 ULTIMATE WIND SPEED (mph)

80 SERVICEABILITY WIND SPEED (mph)

WIND EXPOSURE CATEGORY

0.00 Internal pressure coefficient, gcpi (+/-) (BLDG. "A")

± 0.18 INTERNAL PRESSURE COEFFICIENT, GCpi (+/-) (BLDG. "B")

Open WIND CLOSURE CATEGORY (BLDG. "A")

Enclosed WIND CLOSURE CATEGORY (BLDG. "B")

1.00 SEISMIC IMPORTANCE FACTOR, le

0.096 MAPPED SPECTRAL ACCELERATION FOR SHORT PERIODS, Ss

0.062 MAPPED SPECTRAL ACCELERATION FOR 1-SECOND PERIOD, S1

N/A SEISMIC USE GROUP

B SEISMIC DESIGN CATEGORY

0.0340 SEISMIC RESPONSE COEFFICIENT, Cs

0.102 FIVE PERCENT DAMPED SPECTRAL ACCELERATION FOR SHORT PERIODS, SDS

0.099 FIVE PERCENT DAMPED SPECTRAL ACCELERATION FOR 1-SECOND PERIOD, SD1

D SITE CLASS

3 RESPONSE MODIFICATION FACTORS, R-FRAMES

3 RESP. MOD. FACTORS, R-BRACING (F_SW)

3 RESP. MOD. FACTORS, R-BRACING (B_SW)

NOMINAL WIND SPEED (mph)

SERVICEABILITY WIND RETURN PERIOD (year)

			BUILDING "A"					
0.03	DESIGN BASE SHEAR, W	0.3	LONG. BASE SHEAR (kips)	0.4	TRANS.	BASE	SHEAR	(kips)
			BUILDING "B"					
0.03	DESIGN BASE SHEAR W	1.3	LONG BASE SHEAR (kins)	1.6	TRANS	RASE	SHFAR	(kins)

10 5yr / 12 25yr RAINFALL INTENSITY (in/hr)

FOUIVALENT LATERAL FORCE ANALYSIS PROCEDUR

Systems not specifically detailed for seismic resistance. Seismic Resisting System: Transverse (Rigid Frame)

SPECIAL NOTES

- 1: Girts are not allowed to be cut for field located framed openings unless approved by the engineer. Also, field located framed openings may cause girt sizes to change from original design.
- 2: This building has been designed for a wind speed of <u>145</u> mph. It has been designed assuming any/all glazing will be protected per the <u>IBC 21</u> building code such that the building will remain classified enclosed. The design of the glazing and protection is not part of Metal Building Manufacturer's scope of work.

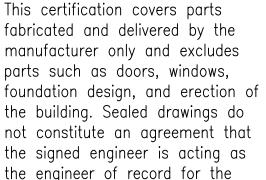


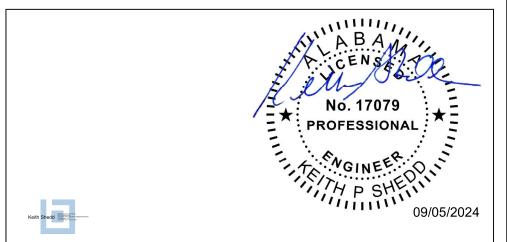


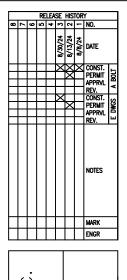


For Customer Service

customerservice@acibuildingsystems.com 662.563.4574 x 2298







Southeastern Erectors, Inc.
D'Iberville, MS 39540
Stella Jones
Bay Minette, AL 36507

CUSTOMER PROJECT

BATESVILLE, MS 38606 (662) 563-4574 (662) 563-1142 (FAX)



ENG BY: JJM
CAD BY: REA
DET BY: REA
CKD BY: RKC

B24-3078

CS1of 2

overall project.

ENGINEERING SEAL

BUILDING SPECIFICATIONS

The building system shown on these drawings has been designed and detailed for the loads and conditions stipulated by the letter of certification, and these drawings.

Any alterations to this building system, removal of any of its components or parts, modification of the intended end—use, modifications in cladding or any other deviations from the original conditions for which the building system was designed may be done only with the written approval of a registered architect and/or a registered professional engineer, as applicable. The metal building manufacturer (MBM) will assume no responsibility for any of the changes mentioned above if performed without prior written approval by the MBM.

This building system has been designed per the MBM's standard design and manufacturing practices, the governing building code, and the applicable editions of the building code referenced AISC, AISI, ASCE, and AWS standards. This building system has also been designed in accordance with all applicable provisions of the latest edition of MBMA Metal Building Systems Manual. In applications including structural steel deck and steel joists, the code referenced editions of applicable SDI and SJI standards, respectively, were also applied.

The MBM does not design or check ventilation or energy conservation systems for the building system supplied and is not responsible for the adequacy of specified ventilation and energy conservation components. The End User should insure that adequate provisions are made for ventilation, condensation, and energy conservation requirements.

The MBM is not responsible for the design, materials and workmanship of the foundation, or the anchorage of the building system to the foundation. Anchor bolt plans prepared by the MBM are intended to show only location, diameter, and projection of anchor bolts required to attach the metal building system to the foundation. The END USER is responsible for engaging the services of a licensed Professional Engineer to perform foundation and foundation anchorage design. The minimum compressive strength of the concrete is assumed to be 3000 psi.

The anchor bolt spacing is based on ACI 318, Section D.8 for cast—in anchors that will not be torqued. The Professional Engineer designing the foundation shall determine the adequate anchor bolt material type and grade, anchor bolt embedment, concrete cover and any anchorage reinforcement to accommodate the given anchor bolt locations, quantity, and diameter.

Unless noted otherwise on the Letter of Certification, the building system by the MBM is exempt from the ASCE 7 stipulated seismic drift limitations. The END USER shall insure that all the interior and exterior attachments and cladding by others are designed to accommodate seismic drift.

The MBM does not investigate the influence of its metal building system on existing buildings or structures. The END USER shall engage services of a licensed Professional Engineer to evaluate whether such buildings and structures are adequate to resist snow drift loads or other conditions as a result of the presence of the Metal Building System. The materials used in fabrication of primary and secondary steel framing members, as well as related accessories are shown below with their corresponding ASTM designations. When the compliance with the building code mandated edition of the AISC Seismic Provisions is required, only materials approved by those provisions are used.

Built-up Section Flanges (Fy = 55 ksi); A529, A572 or A588;
Built-up Section Webs & Connection Plates (Fy = 55 ksi); A1011, A572 or A588;
Hot-rolled W-shapes (Fy = 50 ksi); A992 or A572;
Hot-rolled C and L-shapes (Fy = 50 ksi); A529 or A572;
Hot-rolled Rods (Fy = 55 ksi); A108 or A572;
Cold-formed C, Z, and ES shapes (Fy = 55 ksi); A1011 or A653;
Panels, A792 or A653, Gr. 50 for Ga. 24 and thicker, Gr. 80 for others;
HSS Round; A500 Gr. B (Fy = 42 ksi)
HSS Square/Rectangular; A500 Gr. B (Fy = 46 ksi)
Cables, A475
Eyebolts (Gr. 55); A108, or A572
Washers, A536
Hillside Washers, A48
Structural Bolts, A307 Gr. A, A325 Gr. C, A490 Gr. DH
(used as noted in next section)

Unless noted otherwise and except for crane support system connections, all bolted joints shall be snug—tightened in accordance with the latest edition of Specification for Structural Joints Using ASTM A325, or A490 Bolts (RCSC). All joints in crane support system application shall be pretensioned as required by RCSC. All primary frame bolted connections use A325 bolts, unless noted otherwise. All end—plate connections in cold—formed steel frames use A325 bolts, unless noted otherwise.

All primary structural members have been painted with the minimum of one coat of iron oxide inhibitive primer. All structural steel members have been painted in accordance with Steel Structures Painting Council Specification, SSPC No. 15.

Shop and field inspections and associated fees and expenses are the responsibility of the contractor, unless noted otherwise.

BUYER or CONTRACTOR RESPONSIBILITIES

The BUYER or CONTRACTOR must secure all required approvals and permits for this project from the appropriate agencies in full compliance with all applicable local and state laws and regulations. In accordance with the Sec. 4.4.1 of the latest edition of the AISC Code of Standard Practice and the MBMA Common Industry Practices. Approval of these drawings and calculations (if applicable) constitutes an agreement that the MBM has correctly interpreted the requirements of the contract building drawings, specifications, and all other contractual requirements.

In accordance with Sec. 3.3 of the latest edition of the AISC Code of Standard Practice, where discrepancies exist between drawings provided by the MBM and the drawings provided by the other trades, such as architectural, electrical, plumbing, and others, these drawings provided by the MBM shall govern.

The BUYER or CONTRACTOR is responsible for the erection of the entire building system and all associated work pertaining thereto in accordance with the MBM's "For Construction" drawings. Drawings not marked "For Construction" SHALL NOT be used in the erection of the MBM's building system.

In accordance with Sec. 7.10.3 of the latest edition of the AISC Code of Standard Practice, temporary supports such as guys, braces, falsework, shoring, and other elements necessary to safely erect the building system and prevent structural and other damage to the building system shall be determined and furnished by the erector. The structural building system provided by the MBM is designed for service conditions in accordance with the building code. The BUYER or CONTRACTOR shall erect the system in a manner that insures that the loading conditions on the structure during service are not exceeded in any part of the structure throughout the erection process.

Unless noted otherwise, the MBM shall not be responsible for the design of any elements of this project not part of the structural building system provided by the MBM. The BUYER or CONTRACTOR shall be responsible for taking appropriate steps to insure that such elements are properly structurally designed and constructed.

It is the responsibility of the BUYER or CONTRACTOR to observe and apply all pertinent OSHA and other mandatory safety provisions.

The BUYER or CONTRACTOR is responsible for the inspection of all of the MBM's shipment when received. Any claims of non-received items must be reported to the MBM in writing within 5 business days. In order to maintain the quality guarantee and to qualify for reimbursement, any field modifications of any reported defective item may not be performed without a prior written endorsement by the MBM.

THE MBM shall not be held liable for any claim whatsoever, including, but not limited to, labor charges or consequential damages, resulting from the BUYER or CONTRACTOR/Erector's use of defective or incorrect materials that can be detected by visual inspection.

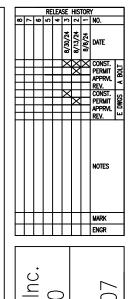
THE MBM is not responsible for material damaged in unloading or for packaged or nested materials, including, but not limited to, fasteners, sheet metal, "C" and "Z" sections, and covering panels that become wet and/or are damaged by water while in the possession of others. Packaged or nested materials that become wet in transit shall be unpacked, unstacked and dried by the BUYER or CONTRACTOR.

With respect to all other building system erection aspects not mentioned above, the BUYER or CONTRACTOR shall comply with the Sec. 6 of the MBMA Common Industry Practices. For any aspects of the erection not covered by the MBMA Common Industry Practices, the provisions of Sec. 7 of the latest edition of the AISC Code of Standard Practice shall apply.

ENGINEERING SEAL

This certification covers parts fabricated and delivered by the manufacturer only and excludes parts such as doors, windows, foundation design, and erection of the building. Sealed drawings do not constitute an agreement that the signed engineer is acting as the engineer of record for the overall project.





Southeastern Erectors, Inc D'Iberville, MS 39540 Stella Jones Bay Minette, AL 36507

CUSTOMER PROJECT

| P.O. BOX 1316 | BATESVILLE, MS 38606 | (662) 563-4574 | (662) 563-1142 (FAX) | STEMS | acibuildingsystems.com



ENG BY: JJM

CAD BY: REA

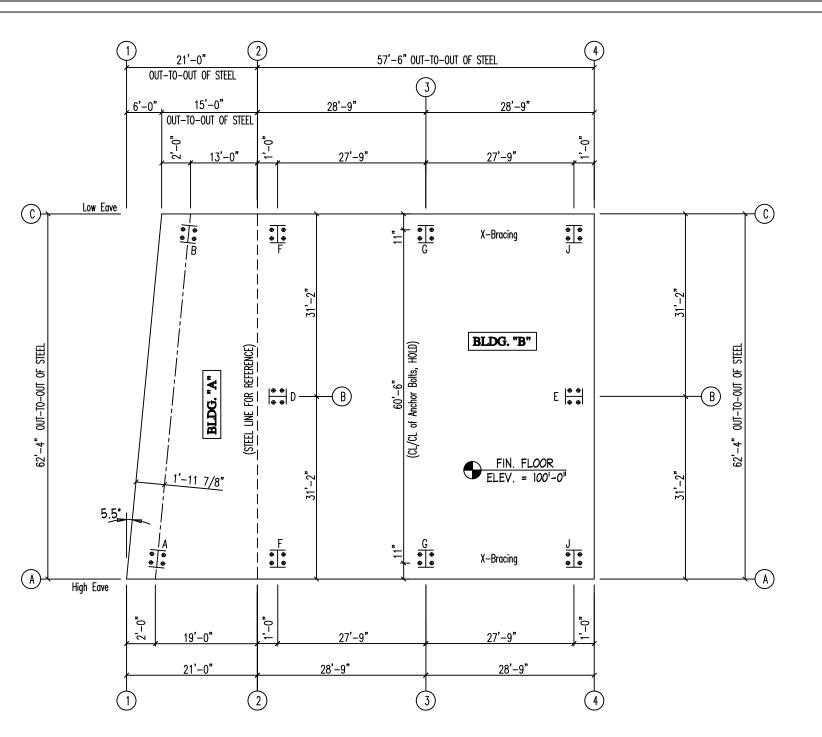
DET BY: REA

CKD BY: RKC

JOB NUMBER:

DWG NUMBER :

B24-3078



Dia (in) Туре Qty Locate 3/4" 2.50 Frame A36 ANCHOR BOLT SUMMARY - BUILDING "B" Proj (in) Dia (in) Locate Туре 3/4" 3/4" 2.50 2.50 ⊕ 8 ⊕ 24 A36 A36 Endwall

No. 17079

PROFESSIONAL

ANCHOR BOLT SUMMARY - BUILDING "A"

MARK ENGR Erectors, Inc. MS 39540 36507 Jones \mathbb{R} Southeastern D'Iberville, N Stella Minette, Bay

CUSTOMER PROJECT



JJM							
REA							
REA							
RKC							
JOB NUMBER :							
B24-3078							

AB1 of 5

ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0"

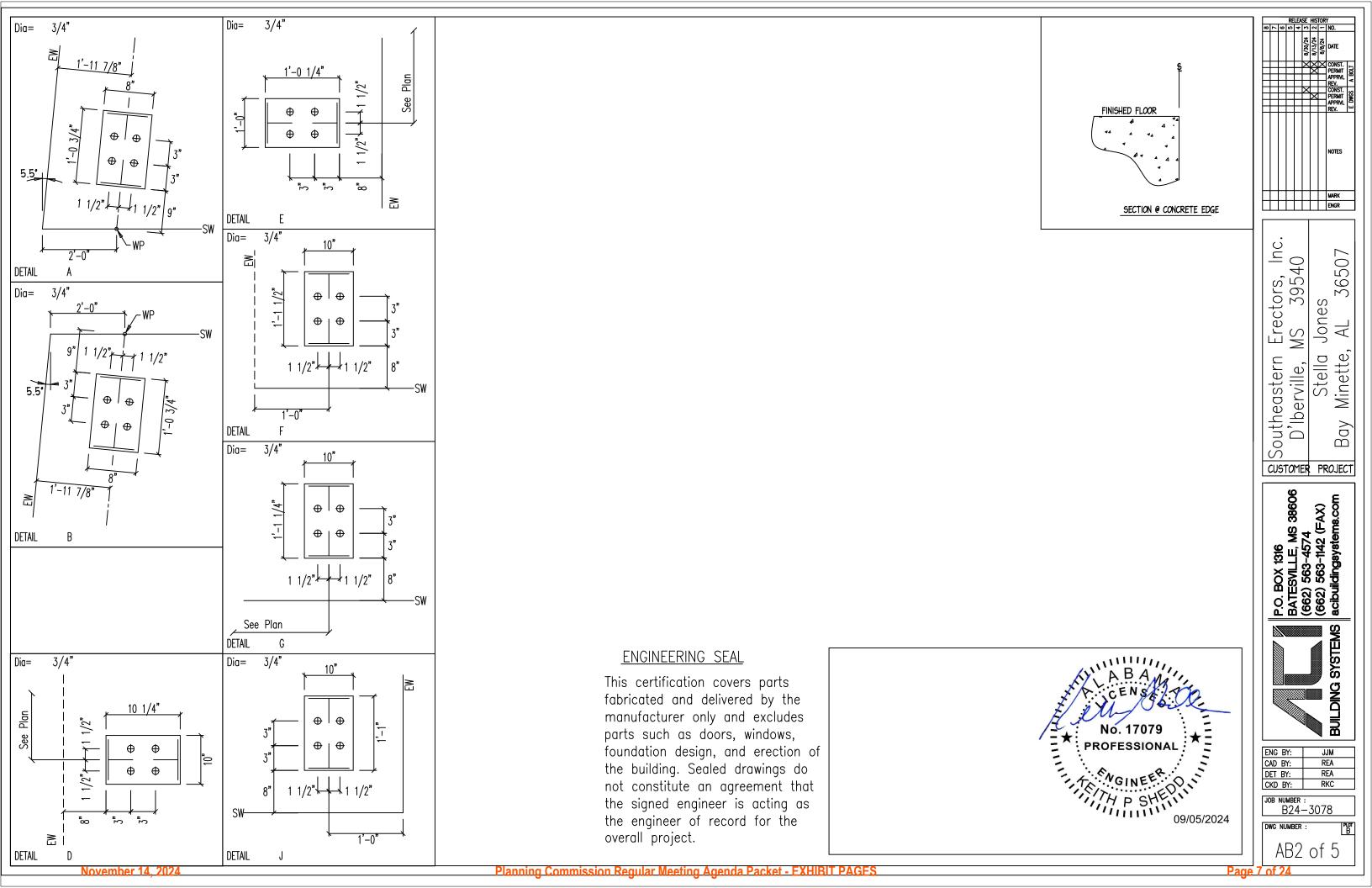
ENGINEERING SEAL

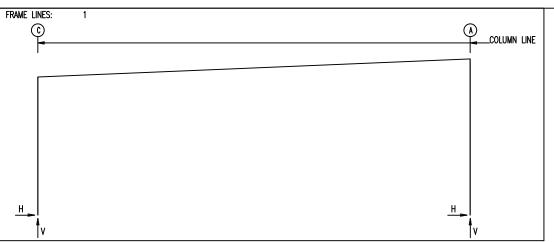
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GENERAL NOTES

- 1. All baseplates @ same elevation unless noted otherwise.
- 2. Anchor bolt diameters are determined base on ASTM F1554, Grade 36 material, unless otherwise noted.
- 3. Anchor bolt design for embedment is not by the metal building manufacturer and must be determined by the foundation engineer.
- 4. See base plate details on AB2 for correct placement of anchor bolts.

09/05/2024





RIGID FR	RAME:		MAXIMUM R	EACTIONS, AN	CHOR BOLT	rs, & BAS	e plates						
Frm Line	Col Line	Load Id	Hmax H	umn_Reacti V Vmax	ons(k) Load Id	Hmin H	V Vmin	Bolt Qty	(in) Dia	Base Width	e_Plate(in) Length	Thick	Elev. (in)
1	С	1 2	2.9 2.9	7.0 8.4	5	-2.3	-7.0	4	0.750	8.000	12.75	0.375	0.0
1	A	5 2	2.9 -3.1	-7.5 8.9	1 5	-3.1 2.9	7.2 -7.5	4	0.750	8.000	12.75	0.375	0.0

RIGID F	RAME:			BAS	IC COLUN	IN REACT	10NS (k)					
Frame Co Line Lir 1 C 1 A	ne Ho		-Dead Vert 1.7 1.9		lateral— Vert 0.4 0.4	Horz 1.8 -1.8	-Live Vert 4.5 4.5	Horz		Wind Horz -3.2 3.9	Vert	Win Horz -3.1 3.9	d_Left2- Vert -9.8 -7.6
Frame Co Line Lir 1 C 1 A	ne Ho	orz -3.2	Right2- Vert -7.1 -10.3	Horz	Vert -13.4	Horz	d_Long2 Vert 3.4 3.4	2—Seisr Horz -0.1 -0.1	mic_Left Vert -0.1 0.1	Seismi Horz 0.1 0.1	ic_Right Vert 0.1 -0.1	-Seisr Horz 0.0 0.0	nic_Long Vert -0.1 -0.1
	ne Ho				lateral— Vert 0.3	 Horz 0.0		Horz	nd_Left1- Vert -7.5	Wind Horz 0.0		Win Horz 0.0	d_Left2- Vert -7.5
Frame Co Line Lir 2 B	ne Ho				d_Long [*] Vert -10.5			2-F2PAT Horz 0.0	_LL_1- Vert 2.0	F2PAT_ Horz 0.0	_LL_2- Vert 1.9		

Building reactions are based on the following building data:

Width (ft) Length (ft) Eave Height (ft) Roof Slope Dead Load (psf)	= =	62.62 22.33 20 0.5:12 2.06	Snow Load (psf) Wind Speed (mph) Wind Code Exposure Closed/Open	= = =	0.0 145 IBC 21 B Open
Collateral Load (psf) Roof Live Load (psf) Frame Live Load (psf)	=	1.00 20.00	Importance - Wind Importance - Seismic Seismic Coeff (Aa)	=	1.00 1.00

NOTES FOR REACTIONS

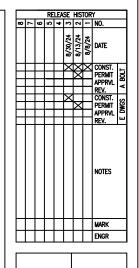
- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- 2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- 4. Reaction values shown are unfactored.
- 5. Loading conditions are:
 - Dead+Collateral+0.75Live+0.45Wind_Long2L Dead+Collateral+0.75Live+0.45Wind_Long2R 0.6Dead+0.6Wind_Left1 0.6Dead+0.6Wind_Right1 0.6Dead+0.6Wind_Long1L 0.6Dead+0.6Wind_Long1R 0.6Dead+0.6Wind_Long2R 0.6Dead+0.6Wind_Right2+0.6Wind_Suction 0.6Dead+0.6Wind_Right2+0.6Wind_Long2L Dead+0.6Wind_Right2+0.6Wind_Suction

BLDG. "A"

ENGINEERING SEAL

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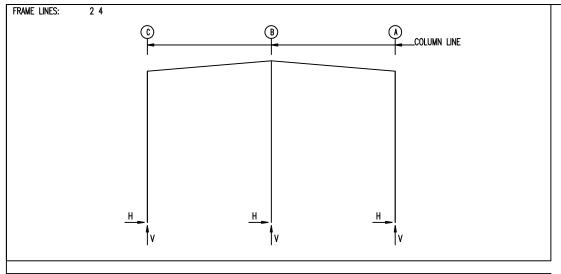
Erectors, Inc. MS 39540 36507 Jones \forall MSSoutheastern Minette, Stella D'Iberville, Bay CUSTOMER PROJECT

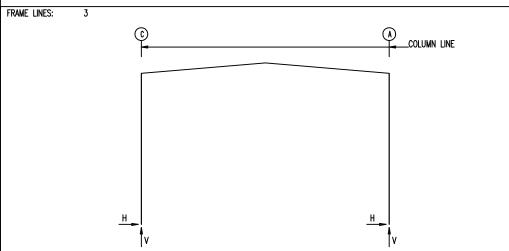
P.O. BOX 1316
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(662) 563-4574
(662) 563-1142 (FAX)
acibuildingsystems.com BUILDING SYSTEMS

ENG BY: JJM CAD BY: REA DET BY: CKD BY: REA RKC

JOB NUMBER : B24-3078

AB3 of 5





RIGID FR	AME:		MAXIMUM R	EACTIONS, AN	CHOR BOL	rs, & Base	PLATES						
Frm Line	Col Line	Load Id	Hmax H	umn_Reaction V Vmax	ons(k) Load Id	Hmin H	V Vmin	- Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Elev. (in)
2	С	3 7	8.6 5.2	1.9 13.7	12 10	-11.0 -8.5	-8.3 -14.0	4	0.750	10.00	13.50	0.500	0.0
2	A	13 6	11.0 -5.1	-8.2 13.6	2 11	-8.6 8.4	1.8 -13.9	4	0.750	10.00	13.50	0.500	0.0
2	В	16 1	14.5 0.0	1.1 21.5	17 11	-13.1 0.0	1.1 -12.2	4	0.750	10.00	10.25	0.500	0.0

IGID FR	AME:		MAXIMUM R	EACTIONS, AN	CHOR BOLT	rs, & Basi	E PLATES						
Frm Line	Col Line	Load Id	Hmax H	umn_Reacti V Vmax	ons(k) Load Id	Hmin H	V Vmin	- Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Elev. (in)
3	C	5 1	9.4 5.9	12.0 21.5	12 14	-13.5 4.9	-10.6 -24.8	4	0.750	10.00	13.25	0.625	0.0
3	A	13 1	13.5 -5.9	-10.6 21.5	4 15	-9.4 -4.9	12.0 -24.8	4	0.750	10.00	13.25	0.625	0.0

GID FR	AME:		MAXIMUM RI	EACTIONS, AN	CHOR BOL	IS, & BASE	E PLATES						
Frm Line	Col Line	Load Id	Coli Hmax H	umn_Reaction V Vmax	ons(k) Load Id	Hmin H	V Vmin	- Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Elev. (in)
4	С	- <u>-</u> 3 9	5.1 2.5	2.5 11.1	12 14	-6.6 2.9	-4.7 -16.8	4	0.750	10.00	13.00	0.500	0.0
4	A	13 8	6.6 -2.5	-4.7 11.1	2 15	-5.1 -2.9	2.5 -16.8	4	0.750	10.00	13.00	0.500	0.0
4	В	18 1	14.8 0.0	-7.4 9.9	17 11	-13.4 0.0	-6.5 -4.4	4	0.750	12.00	12.25	0.375	0.0

RIGII) FRA	ME:		BAS	IC COLUN	IN REAC	TIONS (k)					
Frame	e Colum	n	-Dead-	Col	lateral-		-Live	Win	d_Left1	Wind	_Riaht1-	Win	d_Left2-
Line	Line	Horz	Vert	Horz	Vert	Horz		Horz	Vert	Horz	Vert	Horz	Vert
2	C	0.3	4.3	0.0	3.1	0.7	5.0	-14.5	-27.7	13.8	-4.0	-18.8	-18.1
2	A	-0.3	4.3	0.0	3.1	-0.7	5.0	-13.7	-4.2	14.4	-27.5	-9.5	5.4
2	В	0.0	5.4	0.0	7.5	0.0	8.6	0.0	-25.5	0.0	-25.8	0.0	-14.6
Frame	e Colum	n -Wind		Win				-—-Win			d_Long2	2-Seisr	nic_Left
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert "	Horz	Vert	Horz	Vert
2	С	9.6	5.6	0.0	0.0	0.0	0.0	7.6	-18.0	6.6	-13.0	-0.3	-0.4
2	A	18.7	-17.9	0.0	0.0	0.0	0.0		-13.0		-18.0	-0.3	0.3
2	В	0.0	-15.0	-21.9a	0.0	24.2a	0.0	0.0	-15.4	0.0	-15.4	0.0	0.0
_													
		ın Seismi											
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert				
2	Ç	0.3	0.4	0.0	0.0	0.4	4.9	0.4	0.1				
2	Ā	0.3	-0.3	0.0	0.0	-0.4	0.1	-0.4	4.9				
2	В	0.0	0.0	0.1a	0.0	0.0	4.3	0.0	4.3				
Fram	o Colum	n	_Doad	^^	latoral		مية ا	W:	d Laft1	Wind	Diah+1	w:_	d_Left2-
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	wind Horz	_Kignui- Vert	Horz	Vert
3	C	1.6	7.4	0.3	1.1	4.0	13.0	-20.1		9.8		-24.1	-25.1
3	Ă	-1.6	7. 4 7.4	-0.3	1.1	-4.0			-13.8	20.1	-38.3	-5.8	-23.1 -0.6
J	^	-1.0	7.4	-0.5	1.1	-4.0	13.0	-3.0	-13.0	20.1	-30.3	-5.0	-0.0
Frame	e Colum	n –Wind	Right 2-	Win	d Long	1 — – Wir	d Long	—Seisr	nic Left	Seismi	c Right	-Seisr	nic_Long
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
3	C	5.8	-0.6	6.5	-48.7	5.5	-43.9	-0.3	-0.4	0.3	0.4	0.0	-0.8
3	Ă	24.1	-25.1		-43.9		-48.7		0.4	0.3	-0.4	0.0	-0.8
Frame	e Colum	ın		Col				Win		Wind	_Right1-	Win	d_Left2-
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
4	С	0.2	3.0	0.0	0.3	0.4			-15.0	8.2	-0.9		-11.0
4	A	-0.2	3.0	0.0	0.3	-0.4	3.1	-8.2	-1.0	8.7	-15.0	-5.7	3.1
4	В	0.0	4.5	0.0	0.4	0.0	5.0	0.0	-11.7	0.0	-11.8	0.0	-8.5
_	•		D: 110										
		n –Wind.											
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
4	Ç	5.7	3.2	0.0	0.0	0.0	0.0	4.6	-31.1	4.0	-28.1	-0.2	-0.2
4	A	11.2	-10.9	0.0	0.0	0.0	0.0		-28.1		-31.1	-0.2	0.2
4	В	0.0	− 8.6	−22.4a	0.0	24.7a	0.0	0.0	-8.9	0.0	-8.9	0.0	0.0
Fram	o Colum	n Seismi	o Diaht	_50:00	nio Ion	~ E3DAT	11 1	EZDAT	11 2				
Line	Line	ın seismi Horz	C_Right Vert	-seisi Horz	Nic_Lone Vert	Horz	_LL_1- Vert	Horz	Vert				
4	C	0.2	0.2	0.0	-0.8	0.2	3.0	0.2	0.1				
4	Ă	0.2	-0.2	0.0	-0.8	-0.2	0.1	-0.2	3.0				
4	Ř	0.2	0.0	0.0 0.1a	0.0	0.0	2.5	0.0	2.5				
7	U	0.0	0.0	U. IU	0.0	0.0	2.0	0.0	2.3				
a - 0	Out-Of-I	Plane Hori	zontal La	ad									
				-									

	Line	- Col Line	Horz	Wind — Vert	Horz	smic - Vert	– (lb/ Wind	Seis	No
L_EW	2								_ (
F_SW R_EW	A 4 C	3,4	15.7	20.2	0.6	8.0			(1
B_SW	Ċ	4,3	15.7	20.2	0.6	8.0			V
(h)Rigid	frame at	endwall							

NOTES FOR REACTIONS

- 1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- 2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- 3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- 4. Reaction values shown are unfactored.
- 5. Loading conditions are:

 - Dead+Collateral+Live
 Dead+0.6Wind_Left1
 Dead+0.6Wind_Right1
 Dead+Collateral+0.75Live+0.45Wind_Left1
 Dead+Collateral+0.75Live+0.45Wind_Right1
 Dead+Collateral+0.75Live+0.45Wind_Right2
 Dead+Collateral+0.75Live+0.45Wind_Right2

 - Dead+Collateral+0.75Live+0.45Wind_Long1R Dead+Collateral+0.75Live+0.45Wind_Long2R 0.6Dead+0.6Wind_Left1

- 0.6Dead+0.6Wind_Left1
 0.6Dead+0.6Wind_Right1
 0.6Dead+0.6Wind_Right2
 0.6Dead+0.6Wind_Right2
 0.6Dead+0.6Wind_Long1L
 0.6Dead+0.6Wind_Long2L
 0.6Dead+0.6Wind_Long2L
 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
 0.6Dead+0.6Wind_Fressure+0.6Wind_Long2L
 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
 1.01Dead+1.01Collateral+0.53Seismic_Right

Building reactions are based on the following building data:

Width (ft) = 62.33 Snow Load (psf) = 0.0 Length (ft) = 57.5 Wind Speed (mph) = 145 IBC 21 Eave Height (ft) = 38 Wind Code Roof Slope = 1.0:12 Exposure В Closed/Open Enclosed Dead Load (psf) = 4.22

Importance - Wind = 1.00 Collateral Load (psf) = 1.00 Importance - Seismic = 1.00 Roof Live Load (psf) = 20.00

Seismic Coeff (Aa) = 0.154 Frame Live Load (psf) = 12.00

BLDG. "B"

ENGINEERING SEAL

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BUILDING SYSTEMS ENG BY: REA CAD BY: REA DET BY: CKD BY: JOB NUMBER : B24-3078

lnc.

Southeastern

Erectors, In MS 39540

MS

D'Iberville,

CUSTOMER PROJECT

MS 38606

(662) 563-4574 (662) 563-1142 (FAX) acibuildingsystems.com

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Minette, Stella

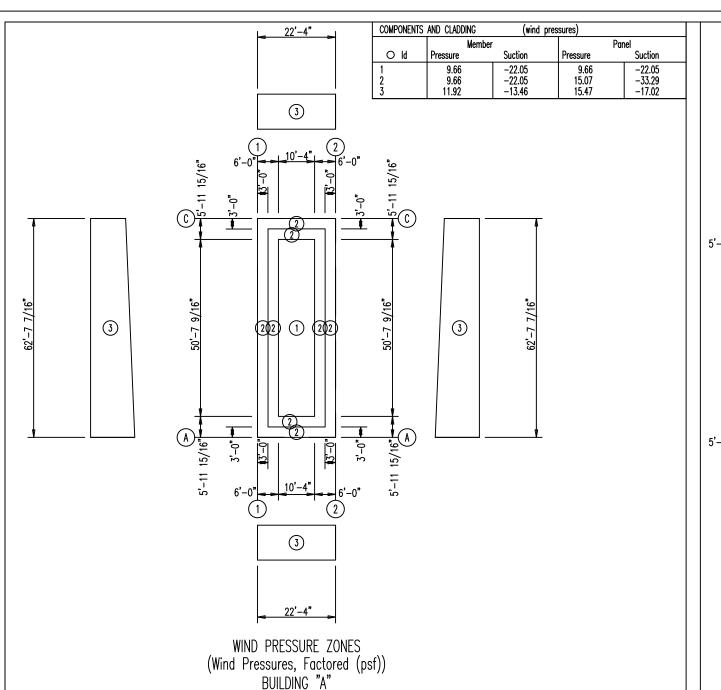
Bay

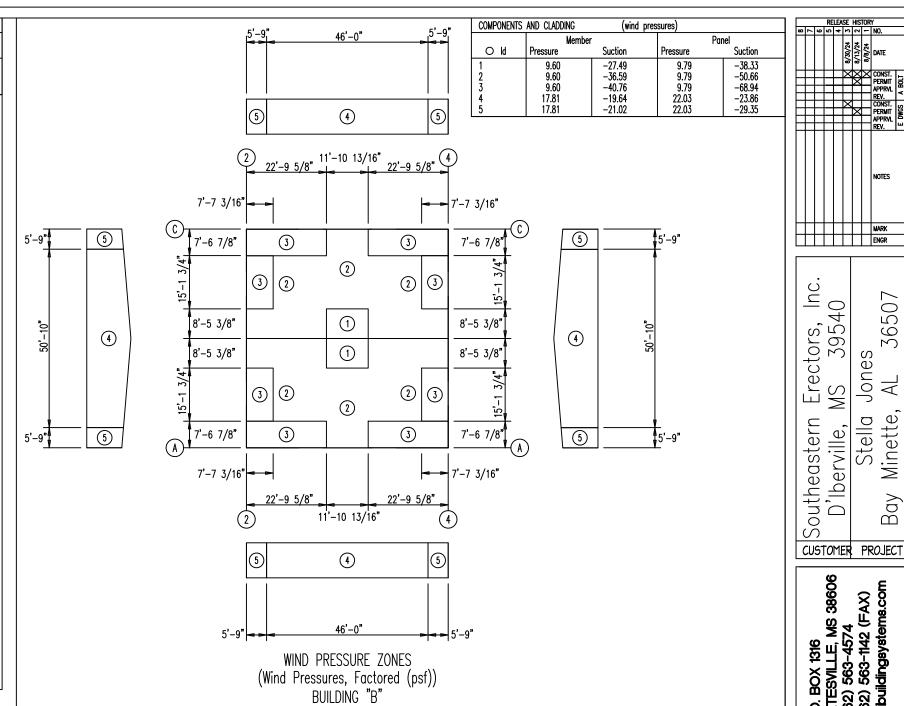
Jones

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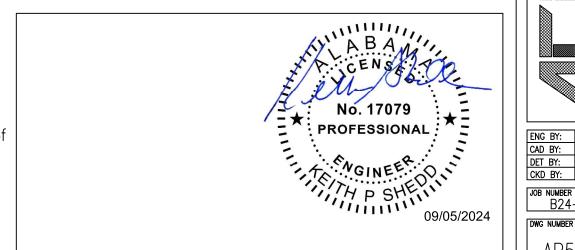
AB4 of 5

November 14, 2024





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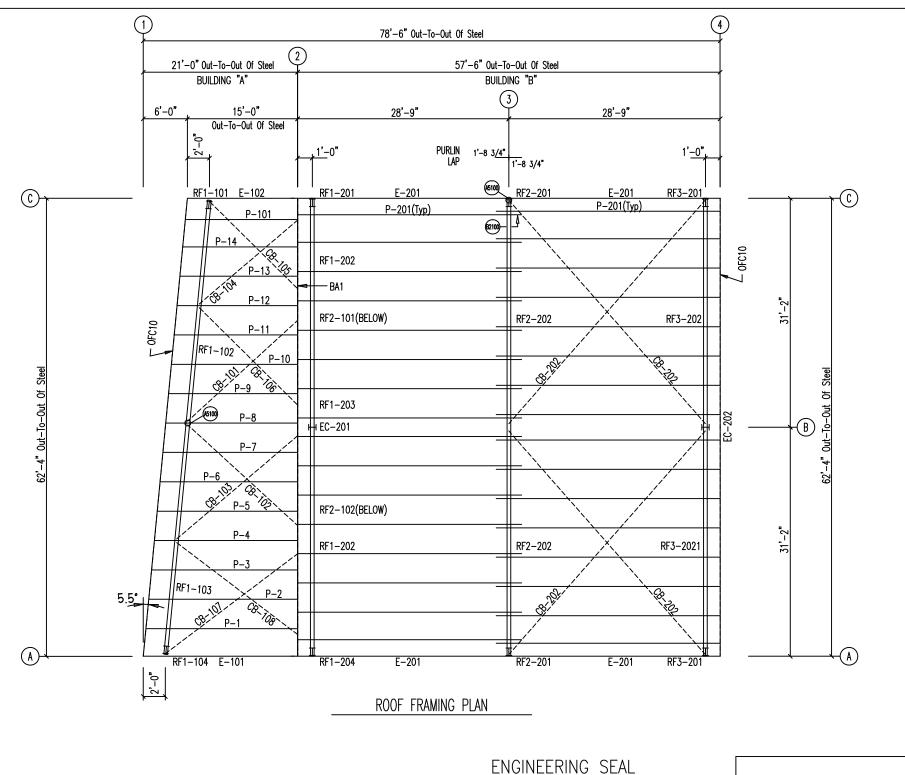
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Bay

Jones \forall

MS



BOLT TABLE LOCATION QUAN TYPE DIA LENGTH 2 A325 1/2" 7 GR. 5 1/2" 2 GR. 5 1/2" Eave Strut to Frame Purlin Lap Connection 1 3/4" 1 1/4" Purlin at EW Rafter 1 1/4"

No. 17079

PROFESSIONAL

MEMBER TA	BLE
ROOF PLAN	
MARK	PART
P-101	10x35Z14
P-1-14	10x35Z14
E-101	10HE1405
E-102	10LE1405
CB-101	1/4" CABLE
CB-102	1/4" CABLE
CB-103	1/4" CABLE
CB-104	1/4" CABLE
CB-105	1'/4" CABLE
CB-106	1/4" CABLE
CB-107	1/4" CABLE
CB-108	1/4" CABLE
	•

LUEUDED TA	DI E
MEMBER TA	
ROOF PLAN	
MARK	PART
P-201	10x25Z12
E-201	10LE1210
CB-202	3/8" CABLE
	•

Inc. Erectors, In MS 39540 \mathbb{R}

Jones Stella Minette, Southeastern D'Iberville,

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Bay

CUSTOMER PROJECT

P.O. BOX 1316 BATESVILLE, MS 38606 (662) 563-4574 (662) 563-1142 (FAX) acibuildingsystems.com



	•
ING BY:	JJM
AD BY:	REA
ET BY:	REA
KD BY:	RKC

JOB NUMBER B24-3078

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THE MAX. WEB LOAD IS 150 LBS FOR EACH MEMBER.

THE LOCATION OF THE LOAD MAY NOT EXCEED I* FROM THE MEMBER WEB HORIZONTALLY AND 1 1/2* ABOVE THE MEMBER FLANGE. THE MAX, FLANGE LOAD IS 50 LBS, FOR EACH MEMBER, THE LOCATION OF THE LOAD MAY NOT EXCEED I[®] FROM THE MEMBER WEB - HOLES FOR HANGER ATTACHMENT MUST NOT BE GREATER THAN 9/16"0 . - THE TOTAL HANGER LOAD SHALL NOT EXCEED THE DESIGN COLLATERAL LOAD EXAMPLE: 5'-0" (MEMBER SPC'G) x 5'-0" (HANGER SPC'G) x 6 PSF (COLLATERAL LOAD) =150 LBS.

WEB ATTACHMENT FLANGE ATTACHMENT

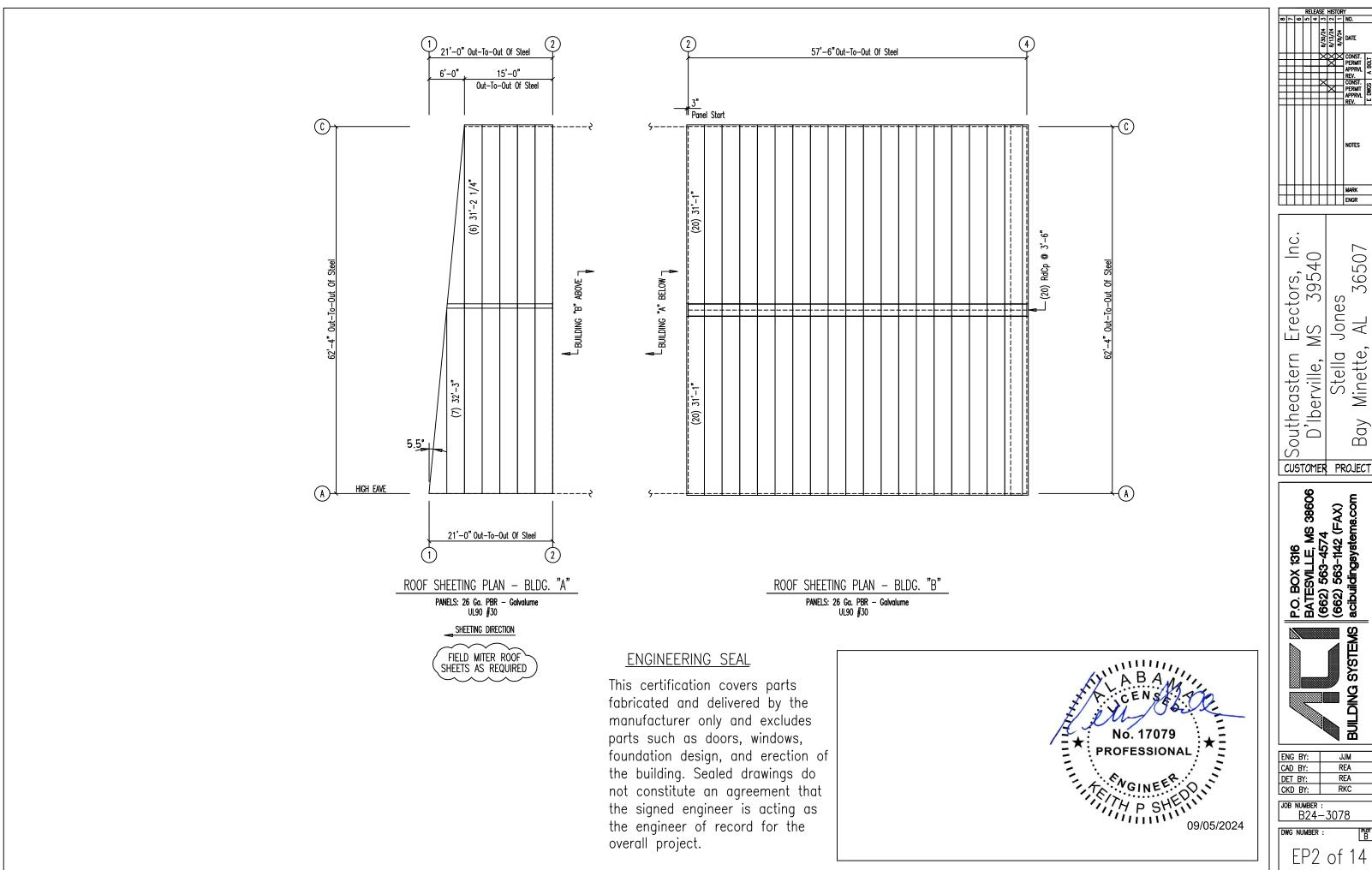
HANGER ASSEMBLY (NOT BY MANUFACTURER CONFIGURATION VARIES

ATTACHMENT FOR COLLATERAL LOADS
ACCEPTABLE CONNECTIONS
November 14, 2024

HANGER ASSEMBLY

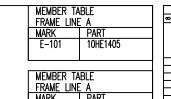
Planning Commission Regular Meeting Agenda Packet - EXHIBIT PAGES

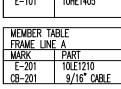
09/05/2024

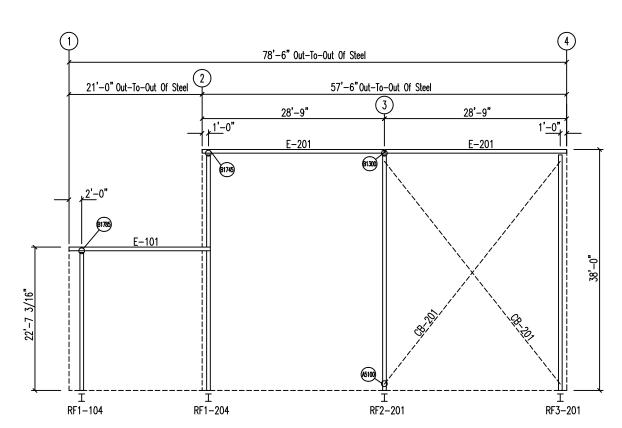


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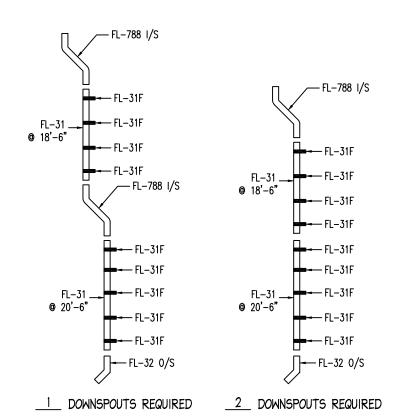
Minette,

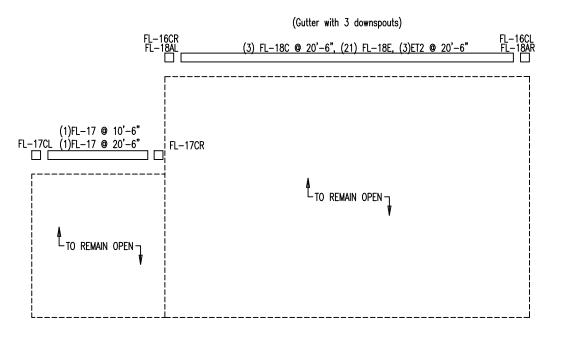






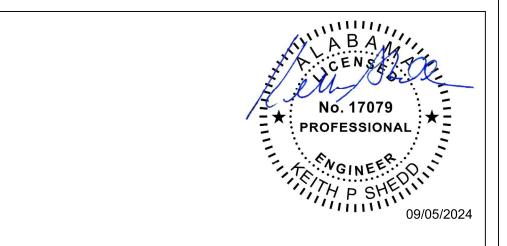
FRONT SIDEWALL FRAMING @ FRAME LINE A

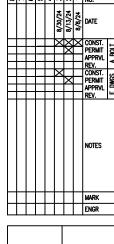




FRONT SIDEWALL SHEETING & TRIM @ FRAME LINE A

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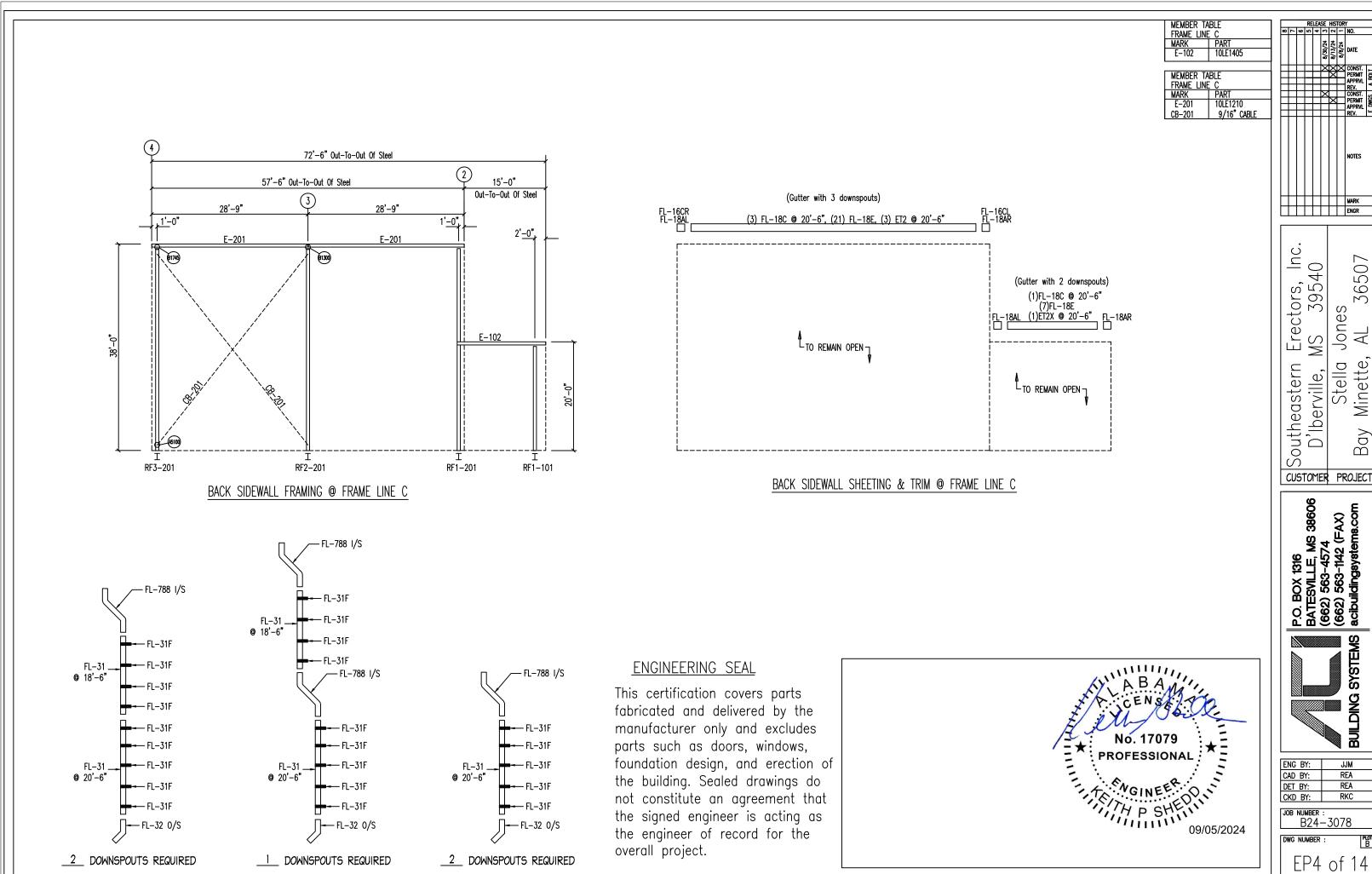
Southeastern Erectors, Inc. D'Iberville, MS 39540 36507 Jones \exists Minette, Stella Bay CUSTOMER PROJECT



ENG BY:	JJM
CAD BY:	REA
DET BY:	REA
CKD BY:	RKC

JOB NUMBER : B24-3078

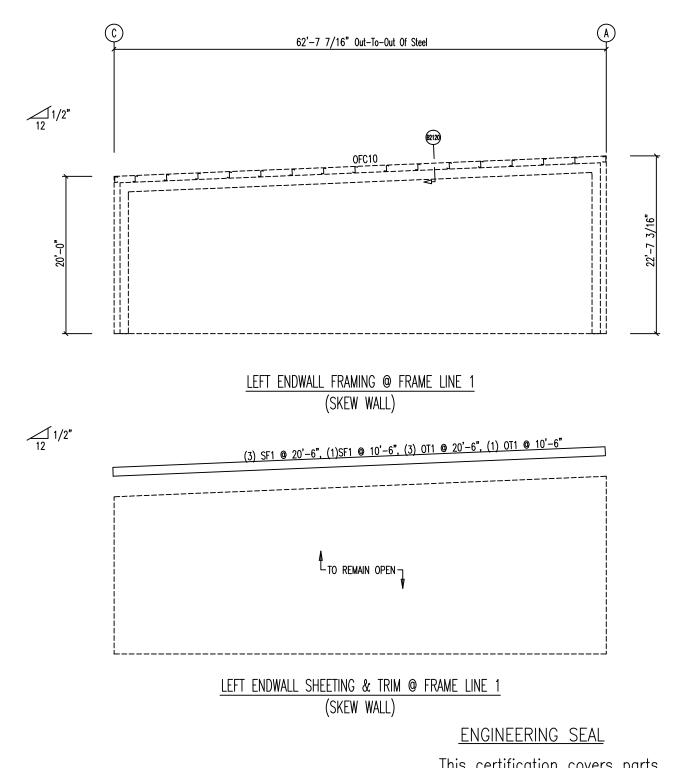
EP3 of 14



Planning Commission Regular Meeting Agenda Packet - EXHIBIT PAGES

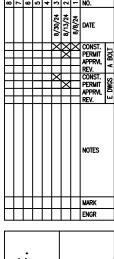
November 14, 2024

<u>'age 14 of 24</u>



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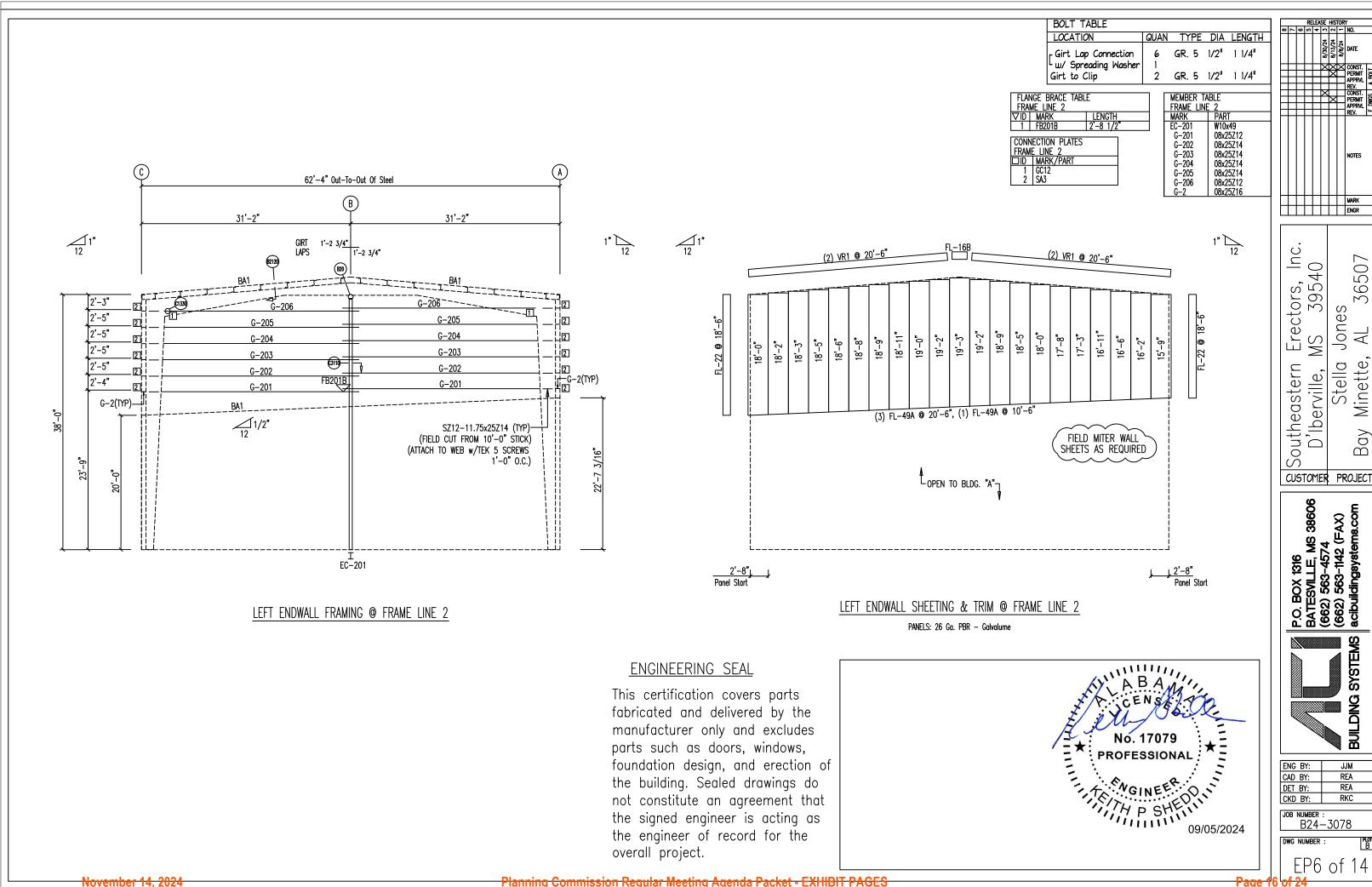
Bay Minette,

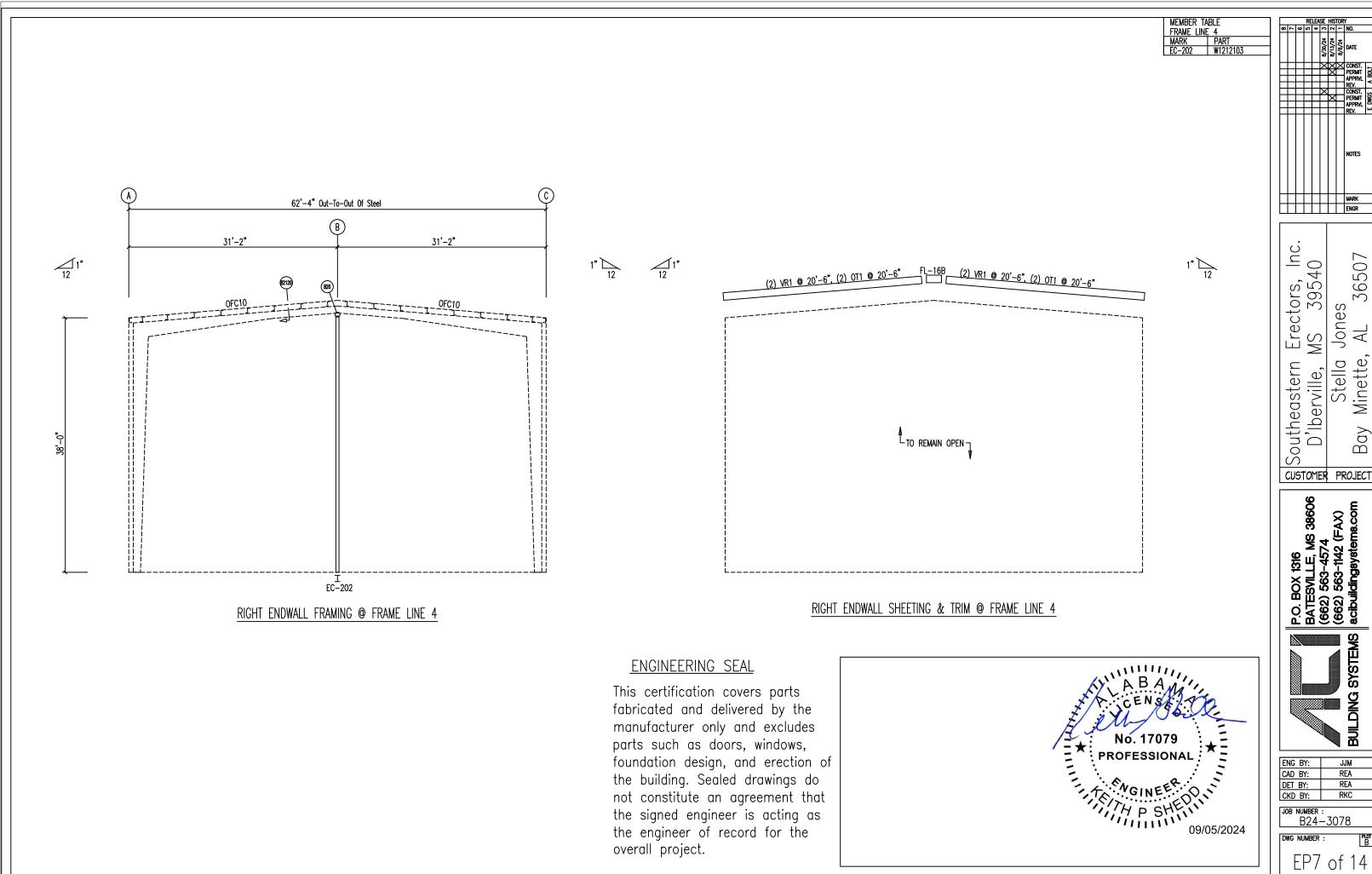


ENG BY: CAD BY: DET BY: CKD BY:

JOB NUMBER : B24-3078

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Minette,

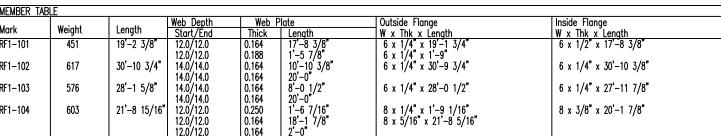
November 14, 2024

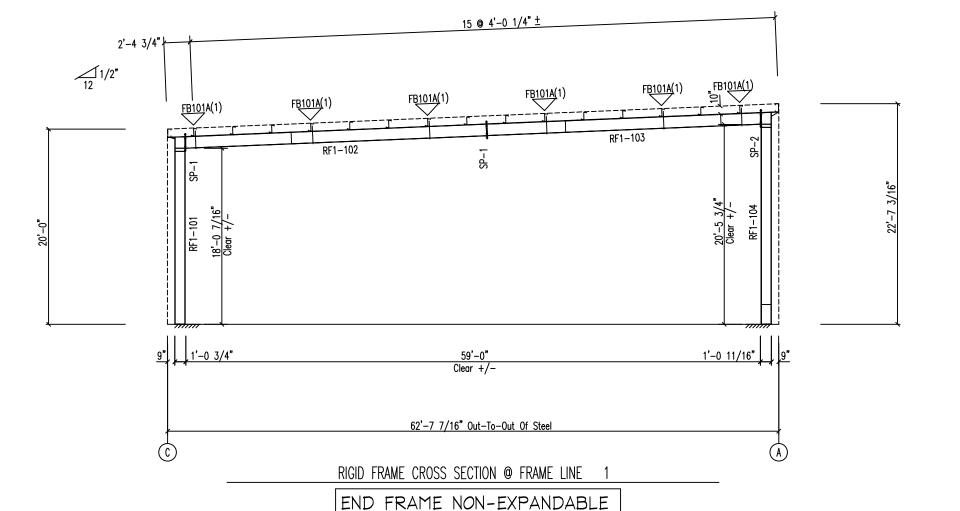
SPLICE PLATES	& B0	LTS							
Splice Mark	Тор	 Bot	Bolt Int	 Dia	Length	Width	Plate Size Thick	Length	7
SP-1	4	4	0	5/8"	2*	6"	1/2"	1'-9 3/4"	┪
SP-2	4	4	0	5′/8"	2 1/4"	8"	5′/8"	1'-9 3′/4"	╛

STIFFENER T	ABLE			
Mark	Stiff Mark	Width	Plate Size Thick	Length
RF1-101 RF1-104	ST1 ST2	2 1/2 3 1/2	1/4" 1/4"	11 7/8" 11 7/8"

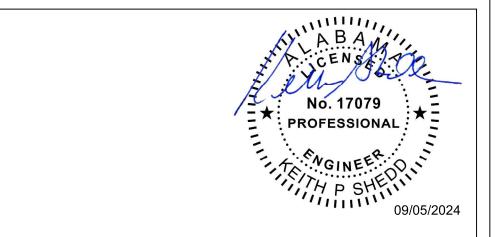
FLANGE BRACES:	Both	Sides(U.N.)
FBxxA(1)		` '
A – L1x1x125		

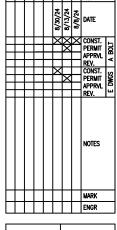
MEMBER TABLE								
Mark	Weight	Length	Web Depth Start/End	Web Plo	ate Lenath	Outside Flange W x Thk x Lenath	Inside Flange W x Thk x Lenath	
RF1-101	451	19'-2 3/8"	12.0/12.0	0.164	17'-8 3/8"	6 x 1/4" x 19'-1 3/4"	6 x 1/2" x 17'-8 3/8"	
RF1-102	617	30'-10 3/4"	12.0/12.0 14.0/14.0	0.188 0.164 0.164	1'-5 7/8" 10'-10 3/8"	6 x 1/4" x 1'-9" 6 x 1/4" x 30'-9 3/4"	6 x 1/4" x 30'-10 3/8"	
RF1-103	576	28'-1 5/8"	14.0/14.0 14.0/14.0	10.164	20'-0" 8'-0 1/2"	6 x 1/4" x 28'-0 1/2"	6 x 1/4" x 27'-11 7/8"	
RF1-104	603	21'-8 15/16"		0.164 0.250 0.164	20'-0" 1'-6 7/16 <u>"</u>	8 x 1/4" x 1'-9 1/16" 8 x 5/16" x 21'-8 5/16"	8 x 3/8" x 20'-1 7/8"	
			12.0/12.0 12.0/12.0	0.164 0.164	18'-1 7/8" 2'-0"	8 x 5/16" x 21'-8 5/16"		





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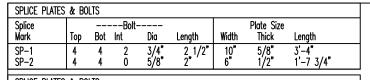
Southeastern Erectors, Inc. D'Iberville, MS 39540 36507 Jones Stella Minette, Bay CUSTOMER PROJECT



ENG BY:	JJM
CAD BY:	REA
DET BY:	REA
CKD BY:	RKC

JOB NUMBER : B24-3078

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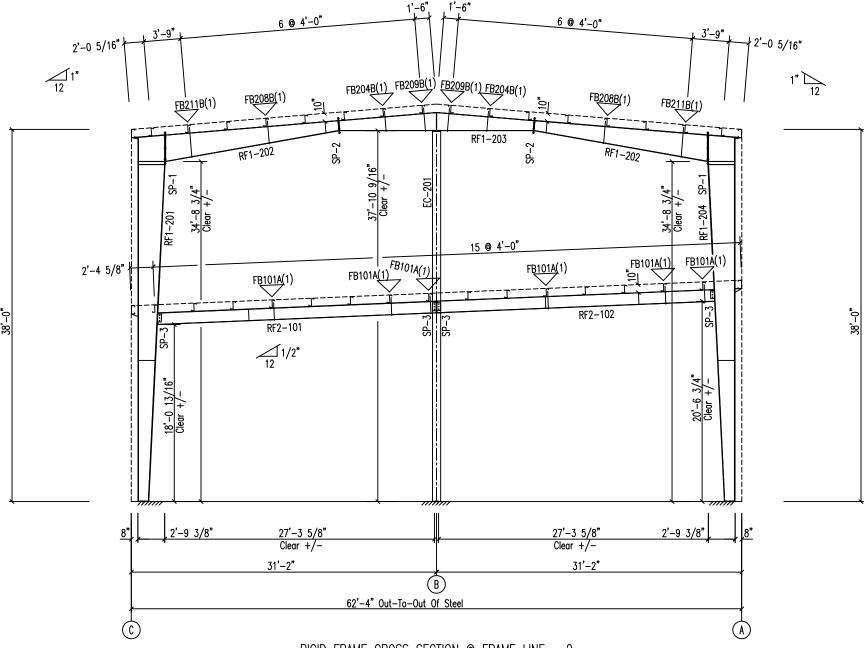
SPLICE PLATES & BOLTS | Top | Bot | Int | Dia | Length | Width | O | O | 3 | 3/4" | 2 | 1/2" | 5" Plate Size Thick Length 3/8" 6"

CAP PLATES

Col		Bolt		Plate Size		
ld	Qty	Dia	Length	Width	Thick	Length
EC-201	4	5/8"	2"	10"	1/2"	10"

STIFFENER TABLE Plate Size Thick Stiff Mark Width Length RF1-201 RF1-203 RF1-204 3 1/2 2 1/2 3 1/2 ST1 ST2 ST3

FLANGE BRACES: Both Sides(U.N.)
FBxxB(1)
A - L1x1x125
B - L2x2x125



RIGID FRAME CROSS SECTION @ FRAME LINE 2

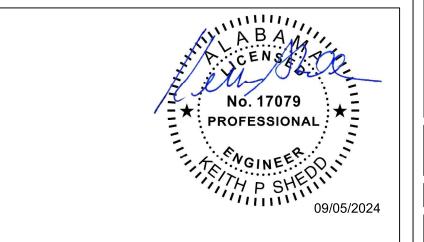
INTERIOR FRAME

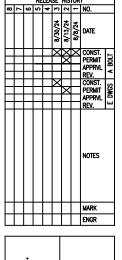
l	MEMBER TAB	LE						
l			l	Web Depth	Web P	'late	Outside Flange	Inside Flange
l	Mark	Weight	Length	Start/Énd	Thick	Length	W x Thk x Length	W x Thk x Length
l	RF1-201	2319	37'-2 5/8"	12.0/20.4	0.164	14'-4 1/2" 20'-0" 3'-0"	10 x 3/4" x 37 ⁻ -1 3/4"	10 x 5/8" x 34'-4 15/16"
l			'	20.4/32.0	0.164	20'-0" ′	10 x 3/8" x 3'-4 5/8"	·
l				32.0/32.0	0.188	3'-0"	' '	
l	RF1-202	500	17'-9 1/4"	32.0/12.0	0.164	17'-10 13/16"	6 x 1/4" x 17'-8 1/8"	6 x 1/4" x 17'-11 3/4"
l	RF1-203	466	17'-9 1/4" 20'-0 3/16"	12.0/21.9	0.164	17'-10 13/16" 9'-11 9/16" 9'-11 9/16" 3'-0"	6 x 1/4" x 10'-0"	l 6 x 1/4" x 9'-5 9/16"
l	=		,	21.9/12.0	0.164	9'-11 9/16"	6 x 1/4" x 10'-0"	6 x 1/4" x 9'-5 9/16"
l	RF1-204	2319	37'-2 5/8"	32.0/32.0	0.188	3'-0"	l 10 x 3/8" x 3'-4 5/8"	10 x 5/8" x 34'-4' 15/16"
l				32.0/20.4	0.164	20'-0"	10 x 3/4" x 37'-1 3/4"	,
l				20.4/12.0	0.164	14'-4 1/2"	,,, .	
l	FC-201	1895	37'-10 5/16"		0.101	,-		

MEMBER TAE	BLE						
Mark	Wainbi	I am mkh	Web Depth	Web P	late	Outside Flange	Inside Flange
	Weight	Length	Start/Énd	Thick	Length	W x Thk x Length	W x Thk x Length
RF2-101	575	28'-4 9/16"	14.0/14.0	0.164	12'-3_5/16"	5 x 1/4" x 28'-4 9/16"	5 x 1/4" x 28'-4 9/16"
		l . '	14.0/14.0	0.164	20'-0"		
RF2-102	534	28'-3 3/16"	14.0/14.0	0.164	10'-3 5/16" 20'-0"	5 x 1/4" x 28'-3 3/16"	5 x 1/4" x 28'-3 3/16"
			14.0/14.0	0.164	20'-0"		

ENGINEERING SEAL

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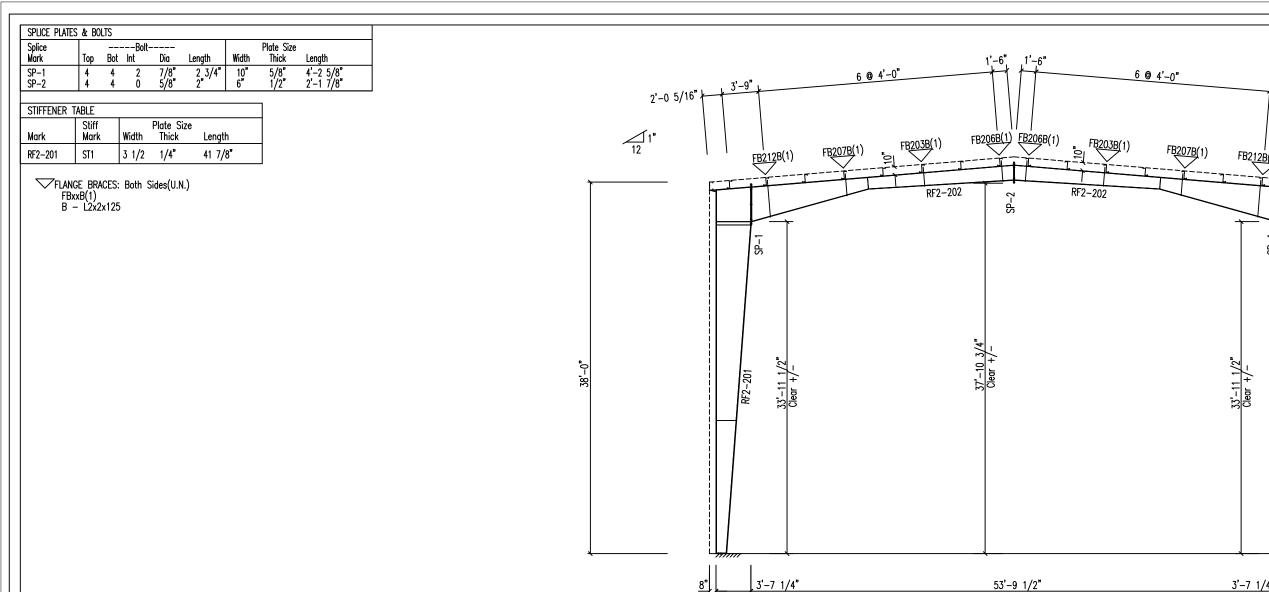
Erectors, Inc. MS 39540 36507 Jones \mathbb{R} Southeastern D'Iberville, N Minette, Stella CUSTOMER PROJECT



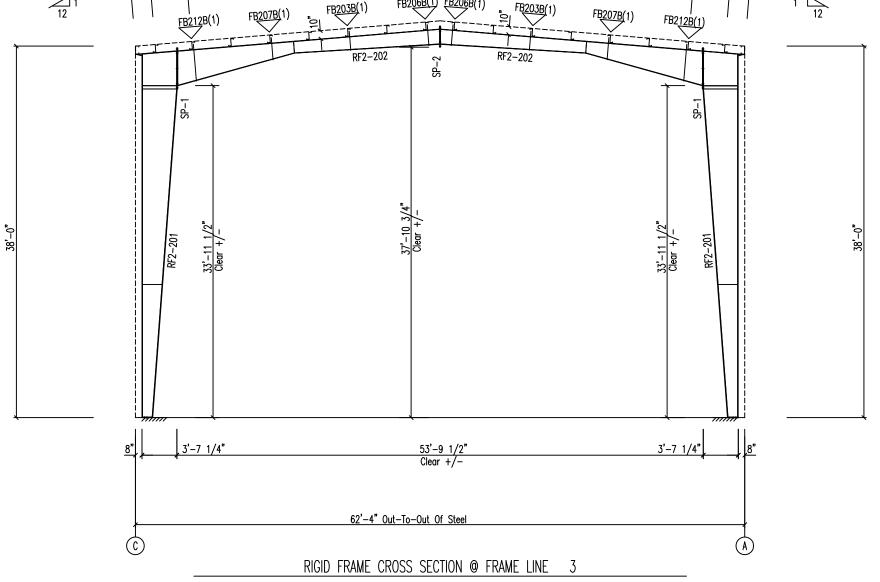
ENG BY:	JJM
CAD BY:	REA
DET BY:	REA
CKD BY:	RKC

JOB NUMBER : B24-3078

EP9 of 14



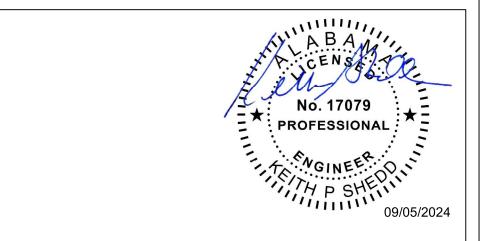
MEMBER TABLE Outside Flange W x Thk x Length 10 x 5/8" x 37'-1 5/8" 10 x 3/8" x 4'-2 1/2" Inside Flange W x Thk x Length 10 x 5/8" x 33'-7 7/8" Web Depth Start/End 12.0/24.1 24.1/42.0 42.0/42.0 42.0/14.0 14.0/18.0 Weight Length 37'-2 5/8' Mark RF2-201 2336 26'-11 7/8" RF2-202

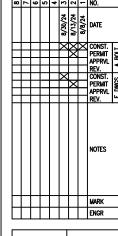


INTERIOR FRAME

ENGINEERING SEAL

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→ 2'-0 5/16"

36507 Jones

Southeastern Erectors, Inc. D'Iberville, MS 39540 Stella D'Iberville, CUSTOMER PROJECT

Minette,

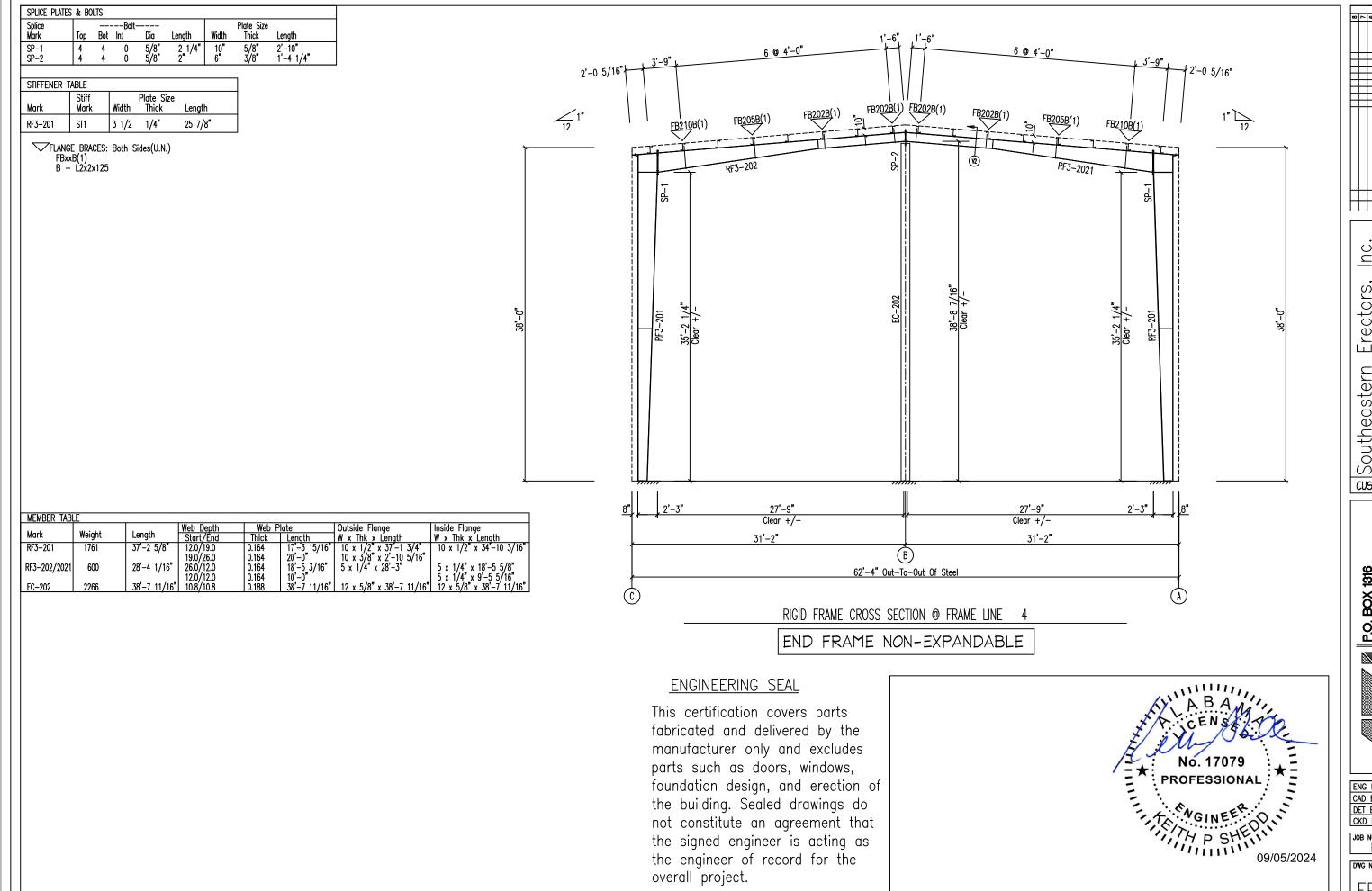
MS 38606



ENG BY:	JJM
CAD BY:	REA
DET BY:	REA
CKD BY:	RKC

JOB NUMBER : B24-3078

DWG NUMBER EP10 of 14



Southeastern Erectors, Inc.
D'Iberville, MS 39540
Stella Jones
Bay Minette, AL 36507

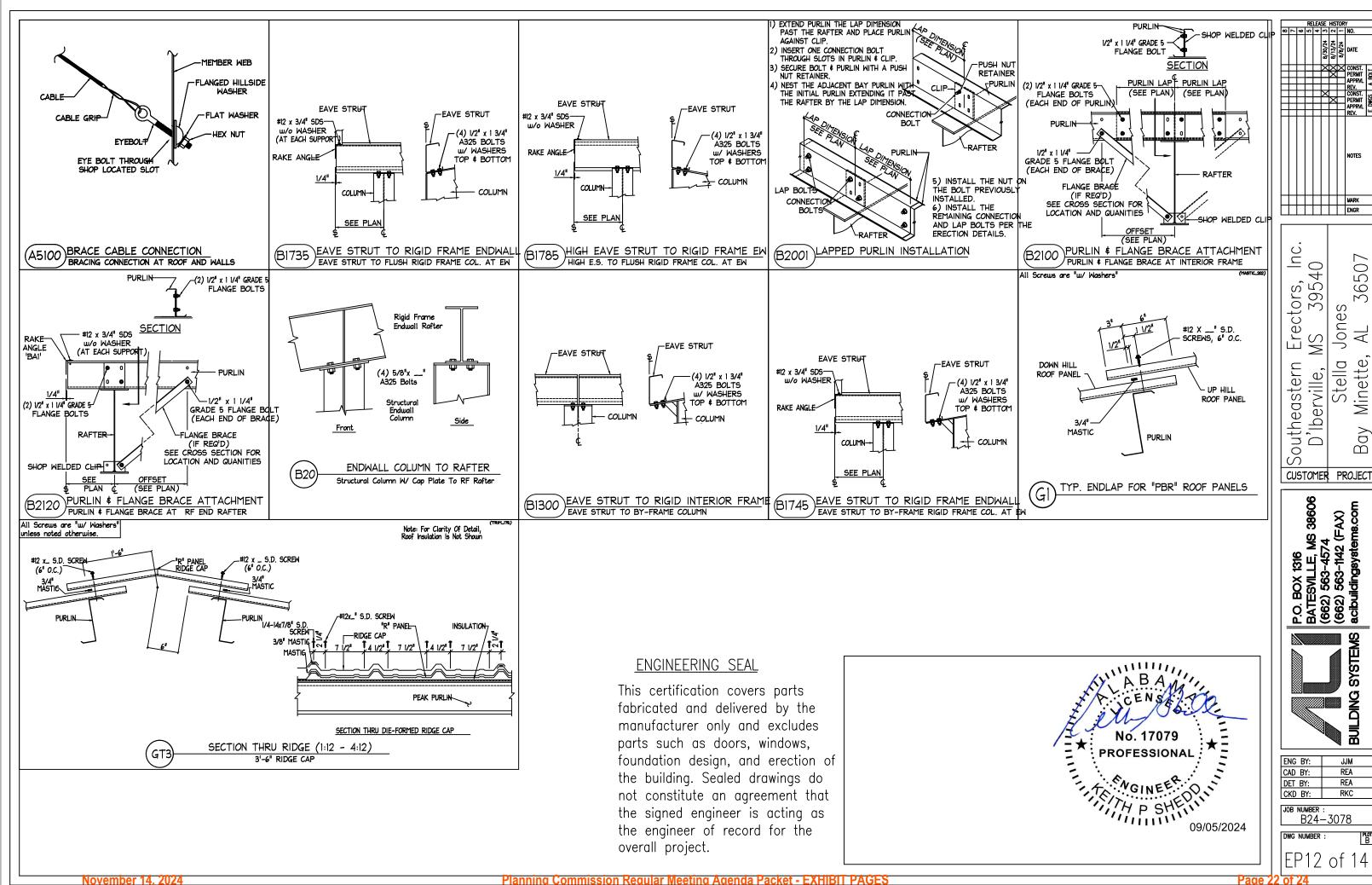
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NG BY: JJM AD BY: RFA

ENG BY:	JJM
CAD BY:	REA
DET BY:	REA
CKD BY:	RKC

JOB NUMBER : B24-3078

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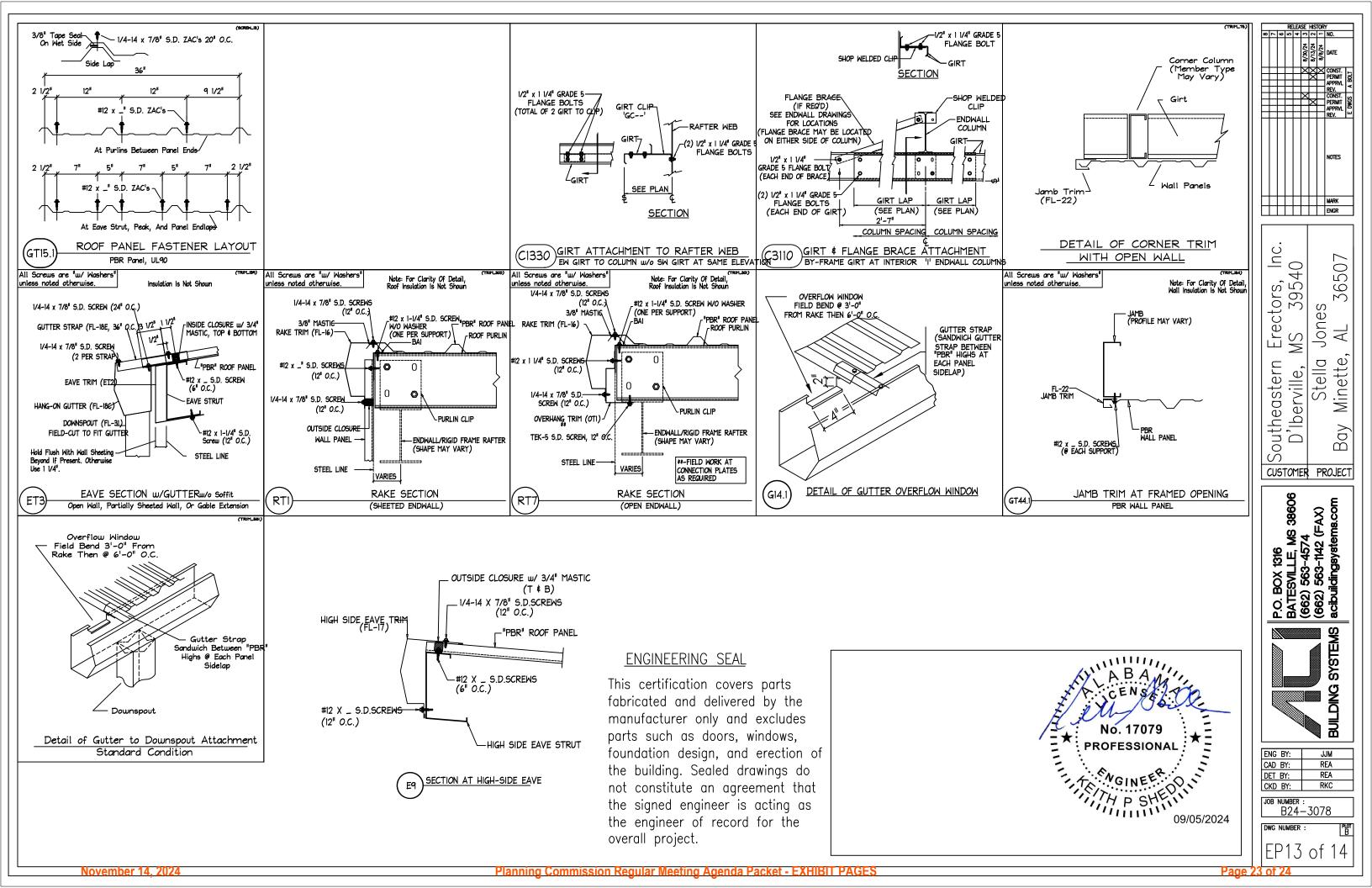


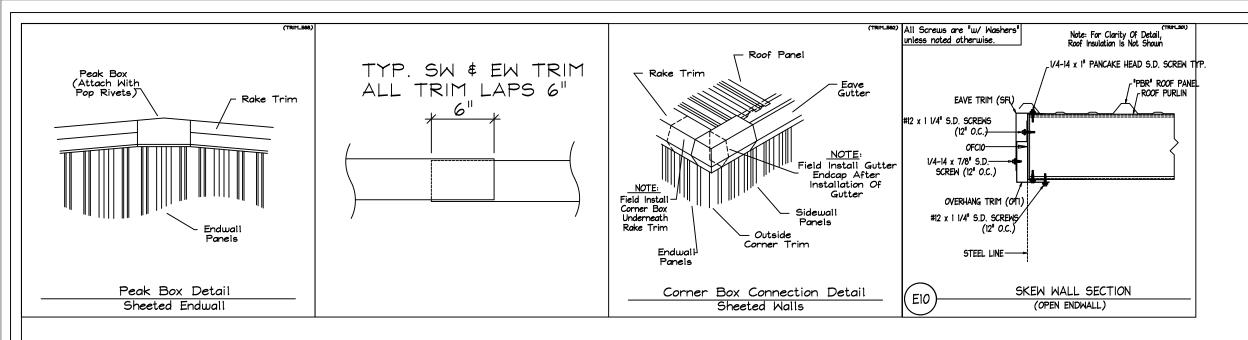
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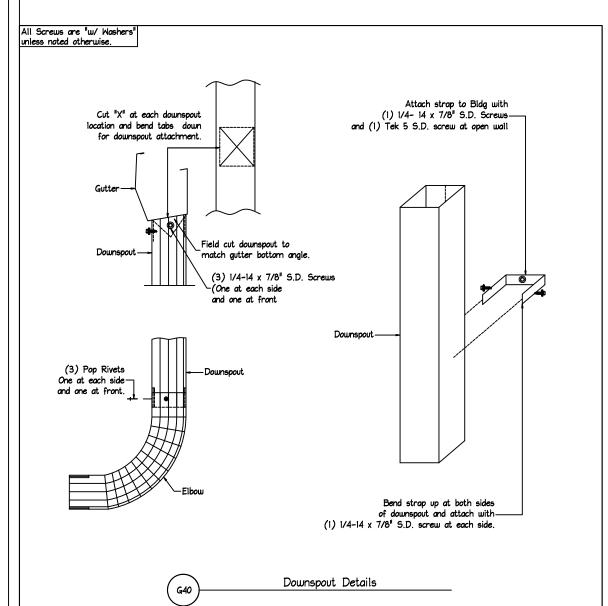
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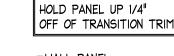
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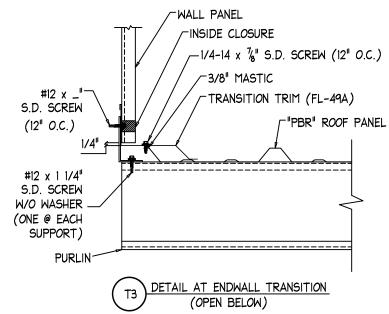
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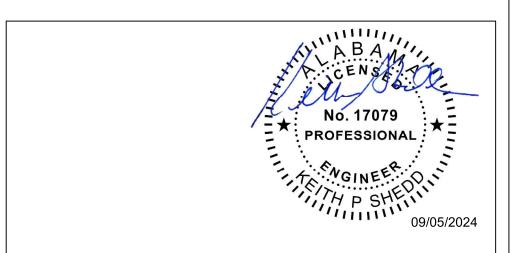


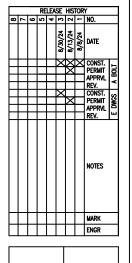






This certification covers parts fabricated and delivered by the manufacturer only and excludes parts such as doors, windows, foundation design, and erection of the building. Sealed drawings do not constitute an agreement that the signed engineer is acting as the engineer of record for the overall project.





36507

Minette,

Bay

Inc. Erectors, In MS 39540 Jones $\stackrel{\mathsf{M}}{\mathsf{M}}$ Stella Southeastern D'Iberville, CUSTOMER PROJECT

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